

TRANSPORTATION IMPACT STUDY

**180 ONTARIO STREET
RESIDENTIAL DEVELOPMENT
TOWN OF COLLINGWOOD
SIMCOE COUNTY**

**PREPARED FOR:
2374515 ONTARIO CORPORATION**

**PREPARED BY:
C.F. CROZIER & ASSOCIATES INC.
70 HURON STREET, SUITE 100
COLLINGWOOD, ON L9Y 4L4**

APRIL 2024

CFCA FILE NO. 2598-6970

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Revision Number	Date	Comments
Rev. 0	April 2024	First Submission

1.0 Executive Summary

2374515 Ontario Corp. retained C.F. Crozier & Associates Inc. (Crozier) to prepare a Transportation Impact Study (TIS) in support of a Zoning By-Law Amendment applications for a proposed residential development at 180 Ontario Street (Subject Property) in the Town of Collingwood.

Development of the Subject Property is proposed to consist of a 4-storey apartment building with 60 residential dwelling units. Access to the site will be provided through a single entrance to Ontario Street. A 6.0 m Right-of-Way (ROW) private driveway is proposed along east side of the building providing access to on-site parking, waste collection bins and the amenity area.

The existing site access at Ontario Street was analyzed as part of this study. Under existing and future background conditions the site operates as a daycare centre and the access operates at a Level of Service (LOS) "B" during the weekday a.m. and p.m. peak hours, with minimal delay and internal queueing of less than one vehicle.

Development of the Subject Property is proposed to generate 15 and 24 total two-way trips during the weekday a.m. and p.m. peak hours, respectively.

Under future total conditions the site access is forecasted to continue operating at a LOS "B" or better during the weekday a.m. and p.m. peak hours with minimal delay and internal queueing of less than one vehicle.

There are opportunities for the Subject Property to incorporate active transportation connections on site and to the wider community. Sidewalks are proposed along the east and south sides of the building connecting to the Ontario Street sidewalk to the north, amenity area to the south and Train Trail to the west. Additionally, bike lanes in both directions are available on Ontario Street as well as a transit stop for the Collingwood Crosstown Route, approximately 30 m from the Subject Property.

The analysis contained within this report was prepared using the Site Plan prepared by Cusimano Architect, dated April 4, 2024. In conclusion, the proposed development can be supported from a transportation perspective.

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2.0 Introduction

2.1 Background

2374515 Ontario Corp. retained C.F. Crozier & Associates Inc. (Crozier) to prepare a Transportation Impact Study (TIS) in support of a Zoning By-law Amendment applications for a proposed residential development at 180 Ontario Street (Subject Property) in the Town of Collingwood. The purpose of the study is to evaluate the transportation-related impacts of the proposed development on the boundary road network and to recommend any mitigation measures, as required. The Subject Property is bound by Ontario Street to the north, residential dwellings to the east, a forested area to the south and an existing public trail to the east. **Figure 1** illustrates the site location.

The Subject Property is approximately 5,200 m² (55,980 ft²) in size and is classified as "Development Deferred Zone (DR)" per Schedule 'A' – Map 21 of the Collingwood Zoning By-Law. There is an existing building currently on site which is operating as a daycare facility with a single entrance from Ontario Street.

2.2 Development Proposal

Development of the Subject Property is proposed to consist of a 4-storey apartment building with 60 residential dwelling units. Access to the site will be provided through a single entrance to Ontario Street shifted to the east of the existing driveway location.

A 6.0 m internal driveway is proposed along east side of the building providing access to on-site parking, waste collection bins and the amenity area.

Figure 2 illustrates the Site Plan prepared by Cusimano Architect, dated April 4, 2024.

2.3 Purpose and Scope

The primary purpose of this Transportation Impact Study is to review the transportation-related impacts of the proposed development, if any, on the boundary road network and to recommend mitigation measures if warranted. This report also addresses other required components specified in the Town's Official Plan regarding active transportation and transit connections.

The study reviews the following main aspects of the proposed development from a transportation engineering perspective:

- Existing, future background and future total operations of the site access.
- Forecasted trip generation and distribution of trips generated by the development.
- Opportunities to encourage alternative transportation (i.e., active transportation and public transportation).

The study has been prepared based on the Terms of Reference circulated to the Town of Collingwood, which received input from the Town's peer reviewer R.J. Burnside. The Terms of Reference communications have been provided in **Appendix A** for reference, as well as responses to the Terms of Reference comments.

3.0 Existing Conditions

3.1 Boundary Road Network

The existing site access at Ontario Street was analyzed as part of this study. Ontario Street is a collector road with a two-lane cross-section, running east-west through the Town of Collingwood. Ontario Street is under the jurisdiction of the Town of Collingwood and includes 2.0 m sidewalks with 1.5 – 2.5 m green boulevards on both sides of the roadway. The collector road also includes dedicated cycling lanes on both sides of the roadway.

Review of the boundary network was scoped to Ontario Street and the proposed site access as the forecasted peak hour trip generated by the development are forecasted to be lower than the existing volumes on site. The trips will also be diffused throughout the external transportation network, minimizing the impact on external intersections. Details regarding the trip generation for the site are outlined in **Section 5.2**.

3.2 Public Transportation Network

Active transportation and transit connections at the Subject Property provide connection to the downtown area of Collingwood as well as surrounding municipalities. **Table 1** outlines the existing active transportation facilities on the boundary road network.

Table 1: Active and Public Transportation Network

Road	Pedestrian Facilities	Cycling Facilities	Transit Routes	Typical Headways
Ontario Street	1.5 metres sidewalk with 1.5 – 2.5 metres grass boulevards on both sides of the roadway. - Train Trail which stems from Collingwood Harbour to Stayner and connects to River trail, Siding trail Pilkington trail and Bay loop trail within the Town.	2.0 m dedicated cycling lane on both sides of the roadway. - Train Trail which stems from Collingwood Harbour to Stayner and connects to River trail, Siding trail Pilkington trail and Bay loop trail within the Town.	Collingwood Crosstown Route	60 mins

Appendix B includes the South Georgian Bay Regional Transit Map for reference. The Crosstown Route bus stops along Ontario Street is located approximately 20 metres east of the site access. The Town began a phased approach to offering On-Demand Transit (ODT) services in June of 2023. An on-demand system will allow greater access to transit services within Town, as services will not be tied to specific routes or schedules.

The multi-use Train Trail is located to the west of the Subject Property and spans from the Town of Collingwood to the community of Stayner, thus providing extensive connectivity within the Georgian Bay area. **Appendix C** contains a Collingwood Trails map.

3.3 Traffic Data

Turning movement count data for the intersection of Ontario Street and the existing site access were collected by Spectrum Traffic Inc. on Wednesday January 31st, 2024, from 6:00 a.m. – 10:00 a.m. and 3:00 p.m. – 7:00 p.m. **Table 2** outlines the calculated peak hour factors at each intersection during each peak hour. **Appendix D** contains the collected traffic data.

Table 2: Intersection Count Peak Hour Factors

Intersection	Count Date	Peak Hour	Peak Hour Factor
Ontario Street & Site Access	Wednesday January 31 st , 2024	Weekday A.M. 8:15 a.m. – 9:15 a.m.	0.89
		Weekday P.M. 3:45 p.m. – 4:45 p.m.	0.87

3.4 Intersection Operations

The 2024 existing site access operations at the study intersection was analyzed based on the volumes illustrated in **Figure 3**. **Appendix E** contains Level of Service definitions. **Appendix F** includes the detailed capacity analysis worksheets. **Table 3** outlines the 2024 site access traffic operations.

Table 3: 2023 Adjusted Existing Traffic Operations

Intersection	Control	Peak Hour	Level of Service	Control Delay	Maximum v/c Ratio	Internal 95 th %ile Queue
Ontario Street & Site Access	Stop	Weekday A.M.	B	10.4 s	0.06 (NB)	1.4 m
		Weekday P.M.	B	11.5 s	0.09 (NB)	2.3 m

Note: The Level of Service of a stop-controlled intersection is based on the delay associated with the critical minor road approach.

The site access is currently operating at LOS “B” during the weekday a.m. and p.m. peak hours, with minimal delay and internal queueing of less than one vehicle.

4.0 Future Background Conditions

4.1 Horizon Years

As outlined in the Terms of Reference established with the Town, the future horizon years of 2028 and 2033, assumed to be the year of full build-out and five years beyond, have been assessed.

4.2 Growth Rate

The Collingwood Transportation Study Update, prepared by R.J. Burnside for the Town in August 2019 used a background growth rate of 0.5% and included the addition of several forecasted developments.

As there are no forecasted developments adjacent to the Subject Property an industry standard annual growth rate of 2% has been applied to the Ontario Street traffic volumes as a conservative assessment.

4.3 Basis of Assessment

To establish the future background traffic volumes, the through volumes on Ontario Street were grown by the growth rate while the existing in and out volumes at the site access were left unchanged as the daycare capacity was assumed to remain consistent. **Figures 4 and 5** outline the 2028 and 2033 future background traffic volumes, respectively.

4.4 Intersection Operations

Table 4 and Table 5 outline the 2028 and 2033 future background traffic operations, respectively. **Appendix E** contains Level of Service definitions. **Appendix F** includes the detailed capacity analysis worksheets.

Table 4: 2028 Future Background Traffic Operations

Intersection	Control	Peak Hour	Level of Service	Control Delay	Maximum v/c Ratio	Internal 95 th %ile Queue
Ontario Street & Site Access	Stop	Weekday A.M.	B	10.6 s	0.06 (NB)	1.4 m
		Weekday P.M.	B	11.7 s	0.09 (NB)	2.4 m

Note: The Level of Service of a stop-controlled intersection is based on the delay associated with the critical minor road approach.

Table 5: 2033 Future Background Traffic Operations

Intersection	Control	Peak Hour	Level of Service	Control Delay	Maximum v/c Ratio	Internal 95 th %ile Queue
Ontario Street & Site Access	Stop	Weekday A.M.	B	10.8 s	0.06 (NB)	1.5 m
		Weekday P.M.	B	12.1 s	0.10 (NB)	2.6 m

Note: The Level of Service of a stop-controlled intersection is based on the delay associated with the critical minor road approach.

The existing site access of the Subject Property is forecast to continue operating at LOS "B" during the weekday a.m. and p.m. peak hours with minimal delay and internal queueing of less than one vehicle.

5.0 Future Total Conditions

5.1 Removal of Existing Volumes

To establish the future volumes at the site access, the existing inbound and outbound volumes to the Subject Property were removed to reflect the removal of the existing use.

5.2 Site Trip Generation

Trip generation for the Subject Property was forecasted using published data from the ITE Trip Generation Manual, 11th Edition. The ITE Trip Generation Manual is a compendium of industry collected trip generation data across North America for a variety of land uses and is used industry wide as a source for trip generation forecasts. Land Use Category 221: Multifamily Housing (Mid-rise) was selected as it defines a mid-rise residential building as four to ten storeys in height.

Table 6 outlines the trip generation for the proposed redevelopment.

Table 6: Site Trip Generation

Use	Roadway Peak Hour	Number of Trips		
		Inbound	Outbound	Total
LUC 221: Multifamily Housing (Mid-rise) (60 Units)	Weekday A.M.	3	12	15
	Weekday P.M.	14	10	24

The proposed redevelopment is expected to generate 15 and 24 total two-way trips during the weekday a.m. and p.m. peak hours, respectively.

5.3 Trip Distribution

Based on 2016 Transportation Tomorrow Survey data, trips generated by existing residential homes in the study zone of the Subject Property distribute approximately 50% to the east and 50% to the west. As such, an equal distribution of generated trips were assigned to the east and west of the Subject Property.

Figures 6 illustrates the trip assignment for the proposed development. **Appendix G** contains the Transportation Tomorrow Survey data for reference.

5.4 Basis of Assessment

The site generated traffic volumes illustrated in **Figure 6** were added to the grown existing through volumes on Ontario Street to determine the future total traffic volumes. **Figures 7 and 8** outline the 2028 and 2033 future total traffic volumes, respectively.

5.5 Intersection Operations

Table 7 and Table 8 outline the 2028 and 2033 future traffic operations, respectively. **Appendix E** contains Level of Service definitions. **Appendix F** includes the detailed capacity analysis worksheets.

Table 7: 2028 Future Total Traffic Operations

Intersection	Control	Peak Hour	Level of Service	Control Delay	Maximum v/c Ratio	Internal 95 th %ile Queue
Ontario Street & Site Access	Stop	Weekday A.M.	A	9.8 s	0.02 (NB)	0.4 m
		Weekday P.M.	B	10.5 s	0.02 (NB)	0.4 m

Note: The Level of Service of a stop-controlled intersection is based on the delay associated with the critical minor road approach.

Table 8: 2033 Future Total Traffic Operations

Intersection	Control	Peak Hour	Level of Service	Control Delay	Maximum v/c Ratio	Internal 95 th %ile Queue
Ontario Street & Site Access	Stop	Weekday A.M.	A	10.0 s	0.02 (NB)	0.4 m
		Weekday P.M.	B	10.7 s	0.02 (NB)	0.5 m

Note: The Level of Service of a stop-controlled intersection is based on the delay associated with the critical minor road approach.

The site access of the Subject Property is forecasted to continue operating at a LOS "B" or better during the weekday a.m. and p.m. peak hours with minimal delay and internal queueing of less than one vehicle. These metrics indicate that the study road network has reserve capacity to accommodate the proposed redevelopment of the site.

6.0 Active Transportation Connections

There are opportunities for the Subject Property to incorporate active transportation connections on site and to the wider community.

6.1 Pedestrian Circulation

As illustrated on the Site Plan, sidewalks are proposed along the east and south sides of the building connecting to the Ontario Street sidewalk to the north, amenity area to the south and Train Trail to the west. **Figure 7** provides illustration of these connections as well as the existing Ontario Street bike lanes in a pedestrian and cycling circulation plan.

6.2 Pedestrian Crossing

Based on aerial imagery there was a painted pedestrian crossing on Ontario Street for the Train Trail which was not repainted after road works between 2016 and 2018. Under existing conditions, a Level 2 Type D pedestrian crossing is recommended per the Ontario Traffic Manual (OTM) Book 15. Growth to the 2033 horizon does not change the recommended crossing type. **Appendix H** contains relevant excerpts from OTM Book 15.

The Town may also wish to implement traffic calming measures per their Traffic Calming Policy, prepared by Tatham Engineering in June 2021. Traffic calming measures such as dynamic speed signs, textured crosswalks, and speed bumps or cushions can be considered within the vicinity of the Train Trail crossing, to reduce speeds and increase driver awareness. **Appendix I** contains excerpts from the Town's Traffic Calming Policy.

7.0 Access Geometry

A single access driveway is proposed for the east side of the Subject Property entrance. The full moves driveway is proposed to be 6.0-7.6 m in width meeting the requirements of the Town of Collingwood Standard Drawing 405. The east and west driveway radii are 8.5 m and 9.0 m, respectively, adhering to the Ontario Provincial Standard Drawings (OPSD) 350.010. It is noted that the eastern curb return ends at the property line.

The location of the driveway is approximately 56m (centreline-to-centreline) from Minnesota Street and 34 m (centreline-to-centreline) from Olde Towne a private roadway on the north side of Ontario Street and the Train Trail on the south side. The locations allows for a 6.0 m drive aisle with parking on both sides.

Ontario Street is relatively straight and flat, providing more than 150 m of sight distance in the east and west directions at the site access. The available site distance is adequate for a roadway with a posted speed of 50 km/h, which require 130 m of available sight distance per the Transportation Association of Canada Geometric Design Guidelines for Canadian Roads.

8.0 Vehicle Maneuverability

A review of vehicle maneuverability through the site was conducted using AutoTurn software. An Aerial Fire Truck, Medium Single Unit Truck and a Simcoe County Waste Collection vehicle were assessed. The waste collection vehicle is required to make a five-point turn to avoid reversing more than 15m through the site. While the body of the vehicle is expected to extend over the curb for this movement, there are no conflicts with the tires. All other design vehicles navigate the site without conflict. **Appendix J** includes the vehicle maneuverability diagrams.

9.0 Conclusions

The analysis contained within this report has resulted in the following key findings:

- The development of the Subject Property is forecast to generate approximately 15 and 24 total two-way trips during the weekday a.m. and p.m. peak hours, respectively.
- Analysis of the future background and total conditions found that the driveway connection to Ontario Street is expected to operate at a Level of Service "B" or better with minimal delay and internal queuing of less than one vehicle in the weekday a.m. and p.m. peak hours.
- Connections to existing sidewalk and trail networks will be provided on site, allowing for increased connectivity with the surrounding network.
- Fire, delivery, and waste collection vehicles can maneuver throughout the site without conflict.

The analysis contained within this report was prepared using the Site Plan prepared by Cusimano Architect, dated April 4, 2024. In conclusion, the proposed development can be supported from a transportation perspective.

Respectfully submitted by,

C.F. CROZIER & ASSOCIATES INC.



Kerianne Hagan, E.I.T.
Engineering Intern, Transportation

C.F. CROZIER & ASSOCIATES INC.



Madeliene Ferguson, P.Eng.
Manager (Planning), Transportation

J:\2500\2598 - 2374515 Ontario Corporation\6970 - 180 Ontario Street\Reports\Transportation\TIS\6970_TIS (April 2024).docx

APPENDIX A

Terms of Reference Correspondence

Kerianne Hagan

From: Diego Bustamante
Sent: February 9, 2024 10:38 AM
To: Kerianne Hagan
Subject: FW: 180 Ontario Street - TIS Terms of Reference

Follow Up Flag: Follow up
Flag Status: Completed

Hi Keri,

FYI.

Regards,

Diego Bustamante, EIT
Engineering Intern, Transportation
DID: 705.434.3421

From: Stefanie Thorne <sthorne@collingwood.ca>
Sent: Wednesday, February 7, 2024 11:05 AM
To: Diego Bustamante <dbustamante@cfcrozier.ca>; Madeleine Ferguson <mferguson@cfcrozier.ca>
Cc: Stuart West <swest@collingwood.ca>; Lindsay Ayers <layers@collingwood.ca>
Subject: 180 Ontario Street - TIS Terms of Reference

Good Morning Diego,

The peer review of the Terms of Reference (TOR) has been completed by R.J. Burnside, for the completion of a Traffic Impact Study (TIS) and Parking Justification Study (PJS). The TOR have been found acceptable, unless identified below (in red) with modifications/clarifications noted in red below:

TRAFFIC IMPACT STUDY (TIS)

- The TIS will be scoped to assess the operations of the existing site access on Ontario Street, which is proposed to remain. **Please confirm the basis for completing a scoped TIS (i.e., access review only), rather than expanding the study to include nearby public road intersections. We note that the site plan proposes that the existing site access is to be shifted, rather than to remain in its current location. Relocation of an existing pole may be required to accommodate this relocation.** The TIS will assess the weekday a.m. and p.m. peak hours.
 - Traffic data collection will occur in January 2024, given the upcoming holiday season.
- We will analyze opening year (assumed 2028), as well as five (2033) years beyond.
- A growth rate of 1.5% will be used. **Please confirm the basis for the assumed growth rate.**
- We kindly request the Town to identify and provide us with any additional nearby developments to be accounted for in the background traffic volume forecasts, if applicable. **Burnside is not aware of any nearby developments and no active developments are shown on the Town's Development map. We defer to the Town**

to confirm if there are any nearby developments that may increase traffic on Ontario Street or if there are any active construction projects that may impact traffic counts in this area.

- The trip generation characteristics of the development will be forecast using ITE Trip Generation Manual, 11th Edition.
 - Trips will be applied to the boundary road network based on Transportation Tomorrow Survey (TTS) data.
- Review the proposed development roadway connections from a geometrics and safety perspective, including turn-lane and signal requirements based on TAC and OTM Book 12 guidelines, as applicable. **Traffic operations (capacity, delay) are to be reviewed using Synchro.**
- Summarize the existing and future active transportation opportunities in the area including sidewalk connectivity, bicycle routes, trail connections to surrounding developments and transit routes. **Also include road crossing requirements and traffic speed/traffic calming requirements on Ontario Street.**
- Prepare a plan that will illustrate active transportation facilities within the Site and any proposed connections to the boundary road network.
- Incorporate vehicle manoeuvring diagrams into the report and summarize findings. **Include AutoTURN swept path analysis for the access and internal to the site for fire trucks, delivery trucks, waste trucks and cars.**

PARKING JUSTIFICATION STUDY (PJS)

In addition to the TIS, we are also seeking confirmation on a proposed scope of work for a parking study to support a reduced parking supply at the site. The Parking Justification Study will review the following items:

By-Law Requirements

A review of the Town of Collingwood Zoning By-Law and Urban Design Manual to determine the required number of vehicle and bicycle parking requirements in relation to the number to be provided on site.

ITE Parking Generation

ITE Parking Generation 5th Edition will be used to calculate the expected parking demand for the development. **Please use ITE Parking Generation 6th Edition, with comparison to the 85th percentile demands.**

Transportation Tomorrow Survey

A review of the TTS data from the Town of Collingwood will determine the expected vehicle ownership rates for the proposed development. **The TTS data that is used for comparison should reflect the type of development (purpose built rental units?, affordable housing?).**

Comparable Municipal Rates

A review of the minimum parking rate requirements of similar municipalities will be undertaken to determine nearby supportable rates.

Proxy Sites

Collection of proxy site data will be used to support the proposed rate. A parking survey will be commissioned at the existing daycare facility to understand existing peak parking and pick-up/drop-off operations. **Please clarify how this data is to be used in the analysis, since it is assumed that the daycare will be replaced by the development.**

In addition, we have reviewed data Crozier has collected in the past and are asking for confirmation of the following sites to be used:

1. 417 Peel Street – Town of Collingwood (Three 4-storey buildings) **Please confirm occupancies, affordable units, number of resident and visitor parking spaces available etc., for comparison to the proposed development. Parking demand data should be separated between resident parking and visitor parking. Please confirm the costs for parking and that it is unbundled from rental rates.**
2. 35-45 Bredin Parkway – Town of Orangeville (One 4-storey building and one 6-storey building) **We don't recommend the use of this site as a proxy site. The development is in close proximity to offsite commercial parking lots and the buildings have a combination of surface and underground parking; these conditions are different than the proposed development.**

A minimum of 3 proxy sites should be reviewed. Please provide two other representative sites, in addition to the 417 Peel Street site. We note that the site plan proposes a parking deficit of 56% from the bylaw requirement; therefore the extent of the PJS should reflect this significant shortfall.

Transportation Demand Management

A review of TDM opportunities for the development, such as transit routes, pedestrian facilities, and proximity to downtown public parking areas, will be undertaken.

If you have any questions, please do not hesitate to contact me.

Sincerely,

Stefanie Thorne, C.Tech.
Engineering Technologist, Engineering Services
705-445-1030 Ext. 4214



Town of Collingwood
545 Tenth Line North, P.O. Box 157
Collingwood ON L9Y 3Z5

sthorne@collingwood.ca | www.collingwood.ca

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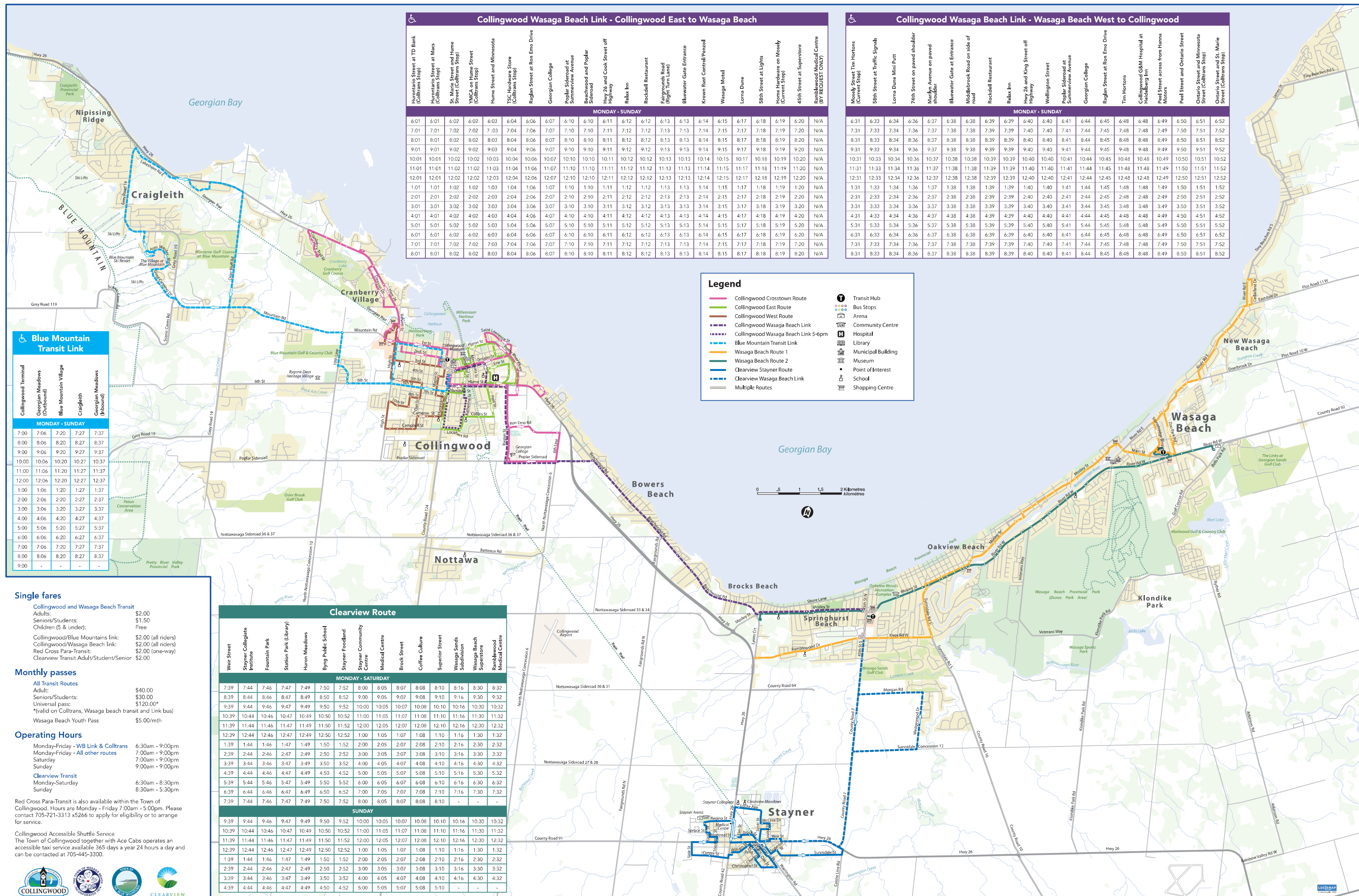
The following responses have been provided to the comments received on the Terms of Reference to provide an ease of review.

Crozier Workplan	Town/Burnside Response	Crozier Response
The TIS will be scoped to assess the operations of the existing site access on Ontario Street, which is proposed to remain. Traffic data collection will occur in January 2024, given the upcoming holiday season.	Please confirm the basis for completing a scoped TIS (i.e., access review only), rather than expanding the study to include nearby public road intersections. We note that the site plan proposes that the existing site access is to be shifted, rather than to remain in its current location. Relocation of an existing pole may be required to accommodate this relocation.	The assessment was scoped to the access as the forecasted peak hour trips generated by the redevelopment are forecasted to be lower than the existing volumes on site. Details regarding the trip generation for the site are outlined in Section 4.4.
We will analyze opening year (assumed 2028), as well as five (2033) years beyond.	-	-
A growth rate of 1.5% will be used.	Please confirm the basis for the assumed growth rate.	The 1.5% growth rate suggestion was a typo. The Collingwood Transportation Study Update, prepared in August 2019 reviewed a background growth rate of 0.5% and the addition of several forecasted developments. As no background developments were identified adjacent to the site, an industry standard annual growth rate of 2% has been applied.
We kindly request the Town to identify and provide us with any additional nearby developments to be accounted for in the background traffic volume forecasts, if applicable.	Burnside is not aware of any nearby developments and no active developments are shown on the Town's Development map. We defer to the Town to confirm if there are any nearby developments that may increase traffic on Ontario Street or if there are any active construction projects that may impact traffic counts in this area.	No background developments were provided by the Town for inclusion.

Crozier Workplan	Town/Burnside Response	Crozier Response
The trip generation characteristics of the development will be forecast using ITE Trip Generation Manual, 11th Edition. Trips will be applied to the boundary road network based on Transportation Tomorrow Survey (TTS) data.	-	-
Summarize the existing and future active transportation opportunities in the area including sidewalk connectivity, bicycle routes, trail connections to surrounding developments and transit routes.	Also include road crossing requirements and traffic speed/traffic calming requirements on Ontario Street.	Discussion of road crossing requirements and traffic calming is included in Section 6.2.
Review the proposed development roadway connections from a geometrics and safety perspective, including turn-lane and signal requirements based on TAC and OTM Book 12 guidelines, as applicable.	Traffic operations (capacity, delay) are to be reviewed using Synchro.	Synchro modelling was completed for existing, future background and future total conditions.
Prepare a plan that will illustrate active transportation facilities within the Site and any proposed connections to the boundary road network.	-	-
Incorporate vehicle manoeuvring diagrams into the report and summarize findings.	Include AutoTURN swept path analysis for the access and internal to the site for fire trucks, delivery trucks, waste trucks and cars.	Please reference the AutoTurn diagrams in Appendix I.

APPENDIX B

South Georgian Bay Regional Transit Map

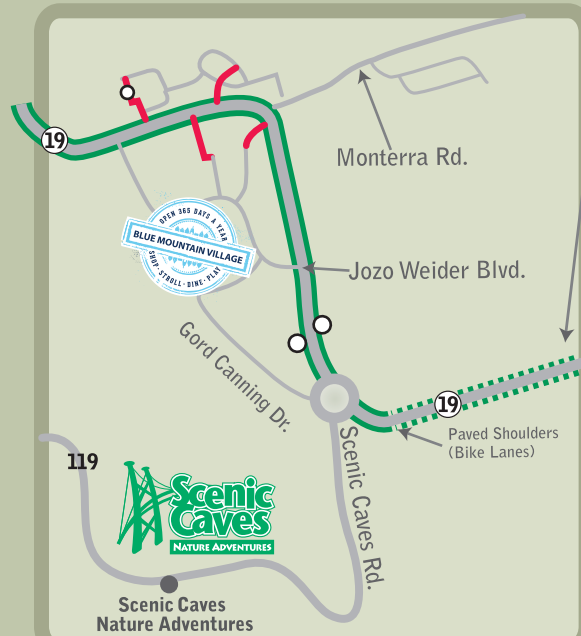


APPENDIX C

Collingwood Trails Map

Interpretive Signs

- 1 Keith Family Dairy
- 2 Collingwood Meat Company
- 3 Manufacturing History
- 4 North-West Ward School
- 5 Collingwood Milling Company
- 6 Smart Bros. Ltd.
- 7 Wetlands
- 8 Birds
- 9 Water/Fish
- 10 Historic Swim
- 11 Walk of History
- 12 Shipbuilder's Park
- 13 North West Mounted Police
- 14 Area History
- 15 Earlier Residents
- 16 Railway Freight Yards
- 17 Sewer & Water History
- 18 Central Park
- 19 Glory Whalen Parkette
- 20 Shannon's Bridge
- 21 Fish Hatchery
- 22 Harbour Plaque
- 23 Bryan Manufacturing
- 24 D.G. Cooper Company Ltd.
- 25 Story of Collingwood 'The Setting'
- 26 Story of Collingwood 1858 to 1908
- 27 Story of Collingwood 1908 to 1958
- 28 Story of Collingwood 1958 to 2008
- 29 Lady Elgin Locomotive
- 30 Cranberry Golf Clubhouse



APPENDIX D

Traffic Data



Turning Movement Count (1 . 180 ONTARIO ST & ONTARIO ST)

Start Time	N Approach NORTH DRIVEWAY						E Approach ONTARIO ST						S Approach 180 ONTARIO ST (ACCESS)						W Approach ONTARIO ST						Int. Total (15 min)	Int. Total (1 hr)	
	Right N-W	Thru N-S	Left N-E	UTurn N-N	Peds N:	Approach Total	Right E-N	Thru E-W	Left E-S	UTurn E-E	Peds E:	Approach Total	Right S-E	Thru S-N	Left S-W	UTurn S-S	Peds S:	Approach Total	Right W-S	Thru W-E	Left W-N	UTurn W-W	Peds W:	Approach Total			
06:00:00	0	0	0	0	0	0	0	4	0	0	0	4	0	0	0	0	0	0	0	1	0	0	0	1	5		
06:15:00	0	0	0	0	0	0	0	5	0	0	0	5	0	0	0	0	0	0	0	2	0	0	0	2	7		
06:30:00	0	0	0	0	1	0	0	3	0	0	0	3	0	0	0	0	0	0	0	3	0	0	0	3	6		
06:45:00	0	0	0	0	1	0	0	8	0	0	0	8	0	0	0	0	0	0	0	4	0	0	0	4	12	30	
07:00:00	0	0	0	0	0	0	0	6	0	0	0	6	0	0	0	0	1	0	0	8	0	1	0	9	15	40	
07:15:00	0	0	0	0	1	0	0	7	1	0	0	8	0	0	0	0	1	0	1	7	0	0	0	8	16	49	
07:30:00	0	0	0	0	0	0	0	15	2	0	0	17	1	0	0	0	1	1	2	9	0	0	1	11	29	72	
07:45:00	0	0	0	0	6	0	0	17	0	0	0	17	0	0	1	0	0	1	2	10	0	0	2	12	30	90	
08:00:00	0	0	0	0	0	0	0	21	4	0	0	25	0	0	2	0	3	2	8	13	0	0	1	21	48	123	
08:15:00	0	0	0	0	1	0	0	40	4	0	0	44	6	0	5	0	0	11	6	20	0	0	1	26	81	188	
08:30:00	0	0	0	0	2	0	0	27	3	0	0	30	2	0	5	0	3	7	7	20	0	0	2	27	64	223	
08:45:00	0	0	0	0	2	0	0	38	3	0	0	41	2	0	7	0	6	9	9	27	0	0	3	36	86	279	
09:00:00	0	0	0	0	2	0	0	39	1	0	0	40	2	0	6	0	2	8	4	23	0	0	2	27	75	306	
09:15:00	0	0	0	0	2	0	0	34	2	0	1	36	2	0	3	0	2	5	0	22	0	0	1	22	63	288	
09:30:00	0	0	0	0	3	0	0	39	1	0	0	40	3	0	1	0	4	4	1	25	0	0	3	26	70	294	
09:45:00	0	0	0	0	5	0	0	41	0	0	0	41	0	0	1	0	2	1	1	33	0	0	2	34	76	284	
BREAK																											
15:00:00	0	0	0	0	3	0	0	35	0	0	0	35	0	0	0	0	5	0	0	32	0	0	2	32	67		
15:15:00	0	0	0	0	6	0	0	48	1	0	1	49	0	0	0	0	2	0	1	29	0	0	3	30	79		
15:30:00	0	0	0	0	2	0	0	33	1	0	0	34	1	0	2	0	4	3	4	32	0	0	5	36	73		
15:45:00	0	0	0	0	2	0	0	37	2	0	0	39	0	0	4	0	4	4	1	37	0	0	3	38	81	300	
16:00:00	0	0	0	0	2	0	0	34	4	0	0	38	4	0	9	0	6	13	10	31	0	0	0	41	92	325	
16:15:00	0	0	0	0	2	0	0	45	4	0	0	49	3	0	13	0	6	16	16	23	0	0	4	39	104	350	
16:30:00	0	0	0	0	1	0	0	21	1	0	0	22	4	0	10	0	3	14	1	47	0	0	0	48	84	361	
16:45:00	0	0	0	0	2	0	0	33	0	0	0	33	2	0	4	0	7	6	0	26	0	0	2	26	65	345	
17:00:00	0	0	0	0	1	0	0	21	0	0	0	21	0	0	0	0	2	0	0	35	0	1	2	36	57	310	
17:15:00	0	0	0	0	4	0	0	39	0	0	0	39	1	0	0	0	3	1	0	23	0	0	1	23	63	269	
17:30:00	0	0	0	0	0	0	0	24	0	0	0	24	0	0	0	0	0	0	0	21	0	0	1	21	45	230	
17:45:00	0	0	0	0	4	0	0	21	0	0	0	21	0	0	0	0	6	0	0	29	0	0	0	29	50	215	
18:00:00	0	0	0	0	0	0	0	14	0	0	0	14	0	0	0	0	11	0	0	15	0	1	1	16	30	188	
18:15:00	0	0	0	0	1	0	0	20	0	0	0	20	0	0	0	0	2	0	0	10	0	0	0	10	30	155	
18:30:00	0	0	0	0	0	0	0	16	0	0	0	16	0	0	0	0	3	0	0	19	0	0	0	19	35	145	
18:45:00	0	0	0	0	6	0	0	14	0	0	0	14	0	0	0	0	16	0	0	15	0	0	0	15	29	124	
Grand Total	0	0	0	0	62	0	0	799	34	0	2	833	33	0	73	0	105	106	74	651	0	3	42	728	1667	-	
Approach%	0%	0%	0%	0%		-	0%	95.9%	4.1%	0%		-	31.1%	0%	68.9%	0%		-	10.2%	89.4%	0%	0.4%		-	-	-	
Totals %	0%	0%	0%	0%		0%	0%	47.9%	2%	0%		50%	2%	0%	4.4%	0%		6.4%	4.4%	39.1%	0%	0.2%		43.7%	-	-	
Heavy	0	0	0	0		-	0	17	0	0		-	0	0	0	0		-	0	12	0	0		-	-	-	
Heavy %	0%	0%	0%	0%		-	0%	2.1%	0%	0%		-	0%	0%	0%	0%		-	0%	1.8%	0%	0%		-	-	-	
Bicycles	-	-	-	-		-	-	-	-	-		-	-	-	-	-		-	-	-	-	-		-	-	-	
Bicycle %	-	-	-	-		-	-	-	-	-		-	-	-	-	-		-	-	-	-	-		-	-	-	



Peak Hour: 08:15 AM - 09:15 AM Weather: Broken Clouds (0.42 °C)

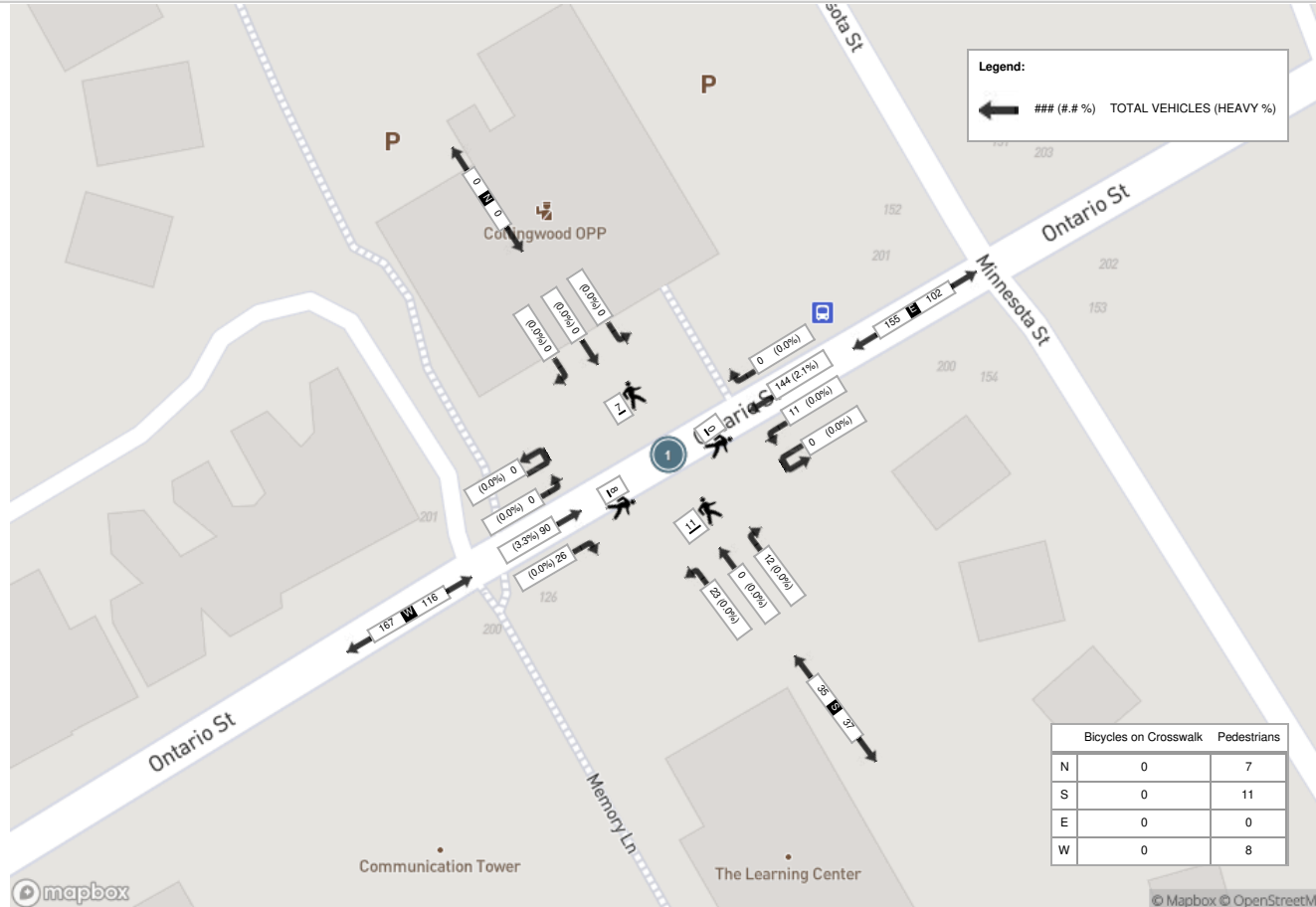
Start Time	N Approach NORTH DRIVEWAY						E Approach ONTARIO ST						S Approach 180 ONTARIO ST (ACCESS)						W Approach ONTARIO ST						Int. Total (15 min)
	Right	Thru	Left	UTurn	Peds	Approach Total	Right	Thru	Left	UTurn	Peds	Approach Total	Right	Thru	Left	UTurn	Peds	Approach Total	Right	Thru	Left	UTurn	Peds	Approach Total	
08:15:00	0	0	0	0	1	0	0	40	4	0	0	44	6	0	5	0	0	11	6	20	0	0	1	26	81
08:30:00	0	0	0	0	2	0	0	27	3	0	0	30	2	0	5	0	3	7	7	20	0	0	2	27	64
08:45:00	0	0	0	0	2	0	0	38	3	0	0	41	2	0	7	0	6	9	9	27	0	0	3	36	86
09:00:00	0	0	0	0	2	0	0	39	1	0	0	40	2	0	6	0	2	8	4	23	0	0	2	27	75
Grand Total	0	0	0	0	7	0	0	144	11	0	0	155	12	0	23	0	11	35	26	90	0	0	8	116	306
Approach%	0%	0%	0%	0%		-	0%	92.9%	7.1%	0%		-	34.3%	0%	65.7%	0%		-	22.4%	77.6%	0%	0%		-	-
Totals %	0%	0%	0%	0%		0%	0%	47.1%	3.6%	0%		50.7%	3.9%	0%	7.5%	0%		11.4%	8.5%	29.4%	0%	0%		37.9%	-
PHF	0	0	0	0		0	0	0.9	0.69	0		0.88	0.5	0	0.82	0		0.8	0.72	0.83	0	0		0.81	-
Heavy	0	0	0	0		0	0	3	0	0		3	0	0	0	0		0	0	3	0	0		3	-
Heavy %	0%	0%	0%	0%		0%	0%	2.1%	0%	0%		1.9%	0%	0%	0%	0%		0%	0%	3.3%	0%	0%		2.6%	-
Lights	0	0	0	0		0	0	139	10	0		149	12	0	23	0		35	26	85	0	0		111	-
Lights %	0%	0%	0%	0%		0%	0%	96.5%	90.9%	0%		96.1%	100%	0%	100%	0%		100%	100%	94.4%	0%	0%		95.7%	-
Single-Unit Trucks	0	0	0	0		0	0	1	0	0		1	0	0	0	0		0	0	2	0	0		2	-
Single-Unit Trucks %	0%	0%	0%	0%		0%	0%	0.7%	0%	0%		0.6%	0%	0%	0%	0%		0%	0%	2.2%	0%	0%		1.7%	-
Buses	0	0	0	0		0	0	2	0	0		2	0	0	0	0		0	0	1	0	0		1	-
Buses %	0%	0%	0%	0%		0%	0%	1.4%	0%	0%		1.3%	0%	0%	0%	0%		0%	0%	1.1%	0%	0%		0.9%	-
Bicycles on Road	0	0	0	0		0	0	2	1	0		3	0	0	0	0		0	0	2	0	0		2	-
Bicycles on Road %	0%	0%	0%	0%		0%	0%	1.4%	9.1%	0%		1.9%	0%	0%	0%	0%		0%	0%	2.2%	0%	0%		1.7%	-
Pedestrians	-	-	-	-	7	-	-	-	-	0		-	-	-	-	11		-	-	-	-	-	8	-	-
Pedestrians%	-	-	-	-	26.9%	-	-	-	-	0%		-	-	-	-	42.3%		-	-	-	-	-	30.8%	-	-
Bicycles on Crosswalk	-	-	-	-	0	-	-	-	-	0		-	-	-	-	0		-	-	-	-	-	0	-	-
Bicycles on Crosswalk%	-	-	-	-	0%	-	-	-	-	0%		-	-	-	-	0%		-	-	-	-	-	0%	-	-



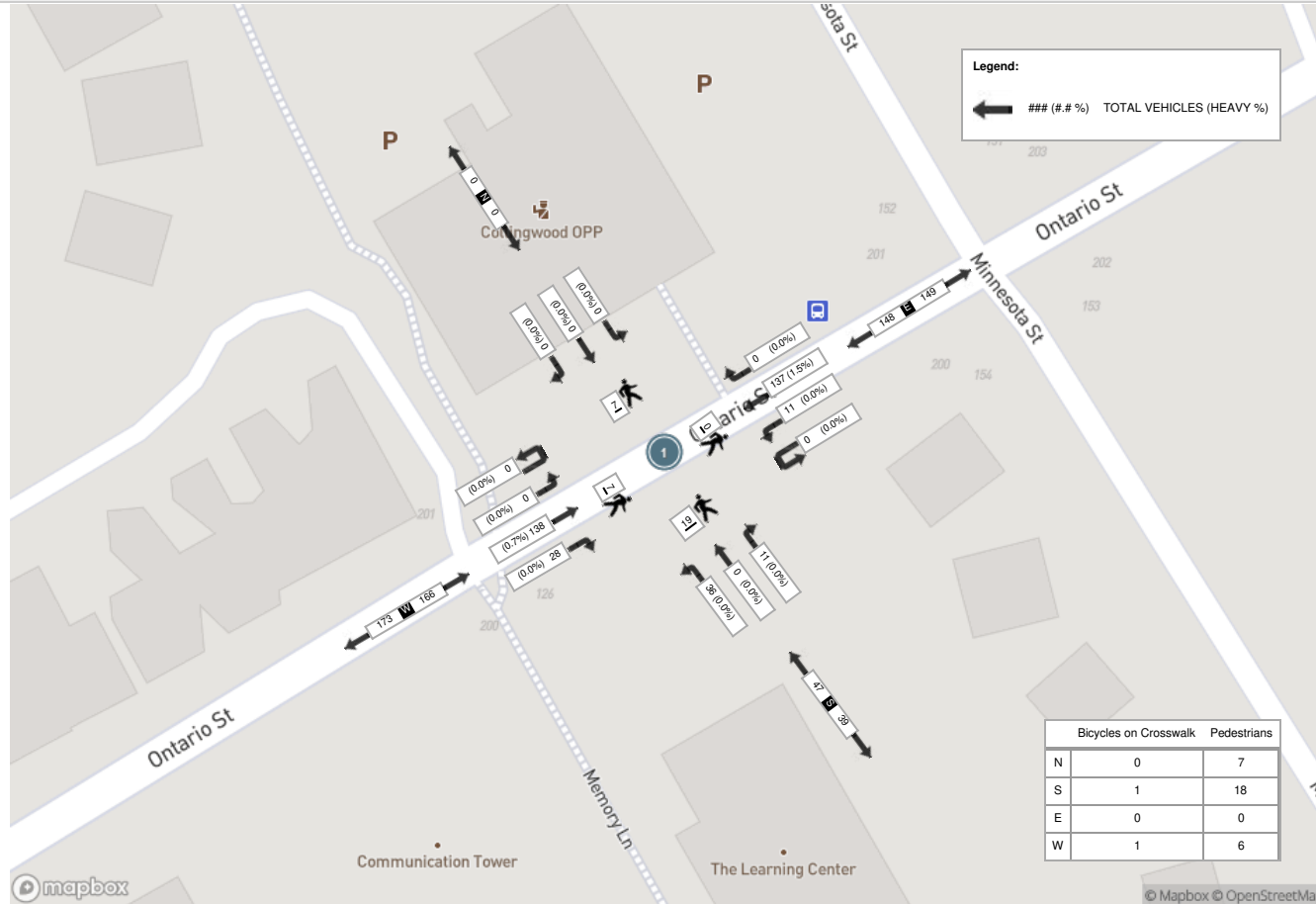
Peak Hour: 03:45 PM - 04:45 PM Weather: Overcast Clouds (2.67 °C)

Start Time	N Approach NORTH DRIVEWAY						E Approach ONTARIO ST						S Approach 180 ONTARIO ST (ACCESS)						W Approach ONTARIO ST						Int. Total (15 min)
	Right	Thru	Left	UTurn	Peds	Approach Total	Right	Thru	Left	UTurn	Peds	Approach Total	Right	Thru	Left	UTurn	Peds	Approach Total	Right	Thru	Left	UTurn	Peds	Approach Total	
15:45:00	0	0	0	0	2	0	0	37	2	0	0	39	0	0	4	0	4	4	1	37	0	0	3	38	81
16:00:00	0	0	0	0	2	0	0	34	4	0	0	38	4	0	9	0	6	13	10	31	0	0	0	41	92
16:15:00	0	0	0	0	2	0	0	45	4	0	0	49	3	0	13	0	6	16	16	23	0	0	4	39	104
16:30:00	0	0	0	0	1	0	0	21	1	0	0	22	4	0	10	0	3	14	1	47	0	0	0	48	84
Grand Total	0	0	0	0	7	0	0	137	11	0	0	148	11	0	36	0	19	47	28	138	0	0	7	166	361
Approach%	0%	0%	0%	0%		-	0%	92.6%	7.4%	0%		-	23.4%	0%	76.6%	0%		-	16.9%	83.1%	0%	0%		-	-
Totals %	0%	0%	0%	0%		0%	0%	38%	3%	0%		41%	3%	0%	10%	0%		13%	7.8%	38.2%	0%	0%		46%	-
PHF	0	0	0	0	0	0	0	0.76	0.69	0		0.76	0.69	0	0.69	0		0.73	0.44	0.73	0	0		0.86	-
Heavy	0	0	0	0		0	0	2	0	0		2	0	0	0	0		0	0	1	0	0		1	-
Heavy %	0%	0%	0%	0%		0%	0%	1.5%	0%	0%		1.4%	0%	0%	0%	0%		0%	0%	0.7%	0%	0%		0.6%	-
Lights	0	0	0	0		0	0	135	11	0		146	11	0	36	0		47	28	136	0	0		164	-
Lights %	0%	0%	0%	0%		0%	0%	98.5%	100%	0%		98.6%	100%	0%	100%	0%		100%	100%	98.6%	0%	0%		98.8%	-
Single-Unit Trucks	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	-
Single-Unit Trucks %	0%	0%	0%	0%		0%	0%	0%	0%	0%		0%	0%	0%	0%	0%		0%	0%	0%	0%	0%		0%	-
Buses	0	0	0	0		0	0	2	0	0		2	0	0	0	0		0	0	1	0	0		1	-
Buses %	0%	0%	0%	0%		0%	0%	1.5%	0%	0%		1.4%	0%	0%	0%	0%		0%	0%	0.7%	0%	0%		0.6%	-
Bicycles on Road	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	1	0	0		1	-
Bicycles on Road %	0%	0%	0%	0%		0%	0%	0%	0%	0%		0%	0%	0%	0%	0%		0%	0%	0.7%	0%	0%		0.6%	-
Pedestrians	-	-	-	-	7	-	-	-	-	0		-	-	-	-	18		-	-	-	-	6		-	-
Pedestrians%	-	-	-	-	21.2%	-	-	-	-	0%		-	-	-	-	54.5%		-	-	-	-	18.2%		-	-
Bicycles on Crosswalk	-	-	-	-	0	-	-	-	-	0		-	-	-	-	1		-	-	-	-	1		-	-
Bicycles on Crosswalk%	-	-	-	-	0%	-	-	-	-	0%		-	-	-	-	3%		-	-	-	-	3%		-	-

Peak Hour: 08:15 AM - 09:15 AM Weather: Broken Clouds (0.42 °C)



Peak Hour: 03:45 PM - 04:45 PM Weather: Overcast Clouds (2.67 °C)



APPENDIX E

Level of Service Definitions

Level of Service Definitions

Two-Way Stop Controlled Intersections

Level of Service	Control Delay per Vehicle (seconds)	Interpretation
A	≤ 10	EXCELLENT. Large and frequent gaps in traffic on the main roadway. Queuing on the minor street is rare.
B	> 10 and ≤ 15	VERY GOOD. Many gaps exist in traffic on the main roadway. Queuing on the minor street is minimal.
C	> 15 and ≤ 25	GOOD. Fewer gaps exist in traffic on the main roadway. Delay on minor approach becomes more noticeable.
D	> 25 and ≤ 35	FAIR. Infrequent and shorter gaps in traffic on the main roadway. Queue lengths develop on the minor street.
E	> 35 and ≤ 50	POOR. Very infrequent gaps in traffic on the main roadway. Queue lengths become noticeable.
F	> 50	UNSATISFACTORY. Very few gaps in traffic on the main roadway. Excessive delay with significant queue lengths on the minor street.

Adapted from Highway Capacity Manual 2000, Transportation Research Board

Appendix F

Capacity Analysis Worksheets

HCM Unsignalized Intersection Capacity Analysis

3: Site Access/OPP Driveway & Ontario Street

2024 Existing AM
02-27-2024

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	0	90	26	11	144	0	23	0	12	0	0	0
Future Volume (Veh/h)	0	90	26	11	144	0	23	0	12	0	0	0
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89
Hourly flow rate (vph)	0	101	29	12	162	0	26	0	13	0	0	0
Pedestrians								11			7	
Lane Width (m)								3.6			3.6	
Walking Speed (m/s)								1.2			1.2	
Percent Blockage								1			1	
Right turn flare (veh)												
Median type		None			None							
Median storage (veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	169			141			312	320	126	322	334	169
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	169			141			312	320	126	322	334	169
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	100			99			96	100	99	100	100	100
cM capacity (veh/h)	1412			1441			627	587	921	612	576	875
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	130	174	39	0								
Volume Left	0	12	26	0								
Volume Right	29	0	13	0								
cSH	1412	1441	701	1700								
Volume to Capacity	0.00	0.01	0.06	0.00								
Queue Length 95th (m)	0.0	0.2	1.4	0.0								
Control Delay (s)	0.0	0.6	10.4	0.0								
Lane LOS		A	B	A								
Approach Delay (s)	0.0	0.6	10.4	0.0								
Approach LOS			B	A								
Intersection Summary												
Average Delay			1.5									
Intersection Capacity Utilization			24.9%		ICU Level of Service				A			
Analysis Period (min)			15									

HCM Unsignalized Intersection Capacity Analysis

3: Site Access/OPP Driveway & Ontario Street

2024 Existing PM
02-27-2024

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	0	138	28	11	137	0	36	0	11	0	0	0
Future Volume (Veh/h)	0	138	28	11	137	0	36	0	11	0	0	0
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87
Hourly flow rate (vph)	0	159	32	13	157	0	41	0	13	0	0	0
Pedestrians								19			7	
Lane Width (m)								3.6			3.6	
Walking Speed (m/s)								1.2			1.2	
Percent Blockage								2			1	
Right turn flare (veh)												
Median type		None			None							
Median storage (veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	164			210			377	384	194	378	400	164
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	164			210			377	384	194	378	400	164
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	100			99			93	100	98	100	100	100
cM capacity (veh/h)	1418			1351			561	536	839	557	525	881
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	191	170	54	0								
Volume Left	0	13	41	0								
Volume Right	32	0	13	0								
cSH	1418	1351	610	1700								
Volume to Capacity	0.00	0.01	0.09	0.00								
Queue Length 95th (m)	0.0	0.2	2.3	0.0								
Control Delay (s)	0.0	0.7	11.5	0.0								
Lane LOS		A	B	A								
Approach Delay (s)	0.0	0.7	11.5	0.0								
Approach LOS			B	A								
Intersection Summary												
Average Delay			1.8									
Intersection Capacity Utilization			26.3%		ICU Level of Service				A			
Analysis Period (min)			15									

HCM Unsignalized Intersection Capacity Analysis

3: Site Access/OPP Driveway & Ontario Street

2028 Future Background AM

02-27-2024

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	0	97	26	11	156	0	23	0	12	0	0	0
Future Volume (Veh/h)	0	97	26	11	156	0	23	0	12	0	0	0
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89
Hourly flow rate (vph)	0	109	29	12	175	0	26	0	13	0	0	0
Pedestrians								11			7	
Lane Width (m)								3.6			3.6	
Walking Speed (m/s)								1.2			1.2	
Percent Blockage								1			1	
Right turn flare (veh)												
Median type		None			None							
Median storage (veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	182			149			334	340	134	342	355	182
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	182			149			334	340	134	342	355	182
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	100			99			96	100	99	100	100	100
cM capacity (veh/h)	1397			1432			607	571	911	592	560	861
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	138	187	39	0								
Volume Left	0	12	26	0								
Volume Right	29	0	13	0								
cSH	1397	1432	683	1700								
Volume to Capacity	0.00	0.01	0.06	0.00								
Queue Length 95th (m)	0.0	0.2	1.4	0.0								
Control Delay (s)	0.0	0.5	10.6	0.0								
Lane LOS		A	B	A								
Approach Delay (s)	0.0	0.5	10.6	0.0								
Approach LOS			B	A								
Intersection Summary												
Average Delay			1.4									
Intersection Capacity Utilization			27.3%		ICU Level of Service				A			
Analysis Period (min)			15									

HCM Unsignalized Intersection Capacity Analysis

3: Site Access/OPP Driveway & Ontario Street

2028 Future Background PM

02-27-2024

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	0	149	28	11	148	0	36	0	11	0	0	0
Future Volume (Veh/h)	0	149	28	11	148	0	36	0	11	0	0	0
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87
Hourly flow rate (vph)	0	171	32	13	170	0	41	0	13	0	0	0
Pedestrians								19			7	
Lane Width (m)								3.6			3.6	
Walking Speed (m/s)								1.2			1.2	
Percent Blockage								2			1	
Right turn flare (veh)												
Median type		None			None							
Median storage (veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	177			222			402	409	206	403	425	177
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	177			222			402	409	206	403	425	177
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	100			99			92	100	98	100	100	100
cM capacity (veh/h)	1403			1337			540	519	826	536	508	866
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	203	183	54	0								
Volume Left	0	13	41	0								
Volume Right	32	0	13	0								
cSH	1403	1337	589	1700								
Volume to Capacity	0.00	0.01	0.09	0.00								
Queue Length 95th (m)	0.0	0.2	2.4	0.0								
Control Delay (s)	0.0	0.6	11.7	0.0								
Lane LOS		A	B	A								
Approach Delay (s)	0.0	0.6	11.7	0.0								
Approach LOS			B	A								
Intersection Summary												
Average Delay			1.7									
Intersection Capacity Utilization			26.8%		ICU Level of Service				A			
Analysis Period (min)			15									

HCM Unsignalized Intersection Capacity Analysis

3: Site Access/OPP Driveway & Ontario Street

2033 Future Background AM

02-27-2024

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	0	108	26	11	172	0	23	0	12	0	0	0
Future Volume (Veh/h)	0	108	26	11	172	0	23	0	12	0	0	0
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89
Hourly flow rate (vph)	0	121	29	12	193	0	26	0	13	0	0	0
Pedestrians								11			7	
Lane Width (m)								3.6			3.6	
Walking Speed (m/s)								1.2			1.2	
Percent Blockage								1			1	
Right turn flare (veh)												
Median type		None			None							
Median storage veh												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	200			161			364	370	146	372	385	200
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	200			161			364	370	146	372	385	200
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	100			99			96	100	99	100	100	100
cM capacity (veh/h)	1376			1417			580	549	898	566	539	841
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	150	205	39	0								
Volume Left	0	12	26	0								
Volume Right	29	0	13	0								
cSH	1376	1417	658	1700								
Volume to Capacity	0.00	0.01	0.06	0.00								
Queue Length 95th (m)	0.0	0.2	1.5	0.0								
Control Delay (s)	0.0	0.5	10.8	0.0								
Lane LOS		A	B	A								
Approach Delay (s)	0.0	0.5	10.8	0.0								
Approach LOS			B	A								
Intersection Summary												
Average Delay			1.3									
Intersection Capacity Utilization			28.1%		ICU Level of Service				A			
Analysis Period (min)			15									

HCM Unsignalized Intersection Capacity Analysis

3: Site Access/OPP Driveway & Ontario Street

2033 Future Background PM

02-27-2024

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	0	165	28	11	164	0	36	0	11	0	0	0
Future Volume (Veh/h)	0	165	28	11	164	0	36	0	11	0	0	0
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87
Hourly flow rate (vph)	0	190	32	13	189	0	41	0	13	0	0	0
Pedestrians								19			7	
Lane Width (m)								3.6			3.6	
Walking Speed (m/s)								1.2			1.2	
Percent Blockage								2			1	
Right turn flare (veh)												
Median type		None			None							
Median storage veh												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	196			241			440	447	225	441	463	196
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	196			241			440	447	225	441	463	196
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	100			99			92	100	98	100	100	100
cM capacity (veh/h)	1381			1316			510	494	806	506	483	845
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	222	202	54	0								
Volume Left	0	13	41	0								
Volume Right	32	0	13	0								
cSH	1381	1316	559	1700								
Volume to Capacity	0.00	0.01	0.10	0.00								
Queue Length 95th (m)	0.0	0.2	2.6	0.0								
Control Delay (s)	0.0	0.6	12.1	0.0								
Lane LOS		A	B	A								
Approach Delay (s)	0.0	0.6	12.1	0.0								
Approach LOS			B	A								
Intersection Summary												
Average Delay			1.6									
Intersection Capacity Utilization			27.7%		ICU Level of Service				A			
Analysis Period (min)			15									

HCM Unsignalized Intersection Capacity Analysis

3: Site Access/OPP Driveway & Ontario Street

2028 Future Total AM
02-27-2024

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	0	97	2	1	156	0	6	0	6	0	0	0
Future Volume (Veh/h)	0	97	2	1	156	0	6	0	6	0	0	0
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89
Hourly flow rate (vph)	0	109	2	1	175	0	7	0	7	0	0	0
Pedestrians								11			7	
Lane Width (m)								3.6			3.6	
Walking Speed (m/s)								1.2			1.2	
Percent Blockage								1			1	
Right turn flare (veh)												
Median type	None			None								
Median storage (veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	182			122			298	305	121	301	306	182
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	182			122			298	305	121	301	306	182
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	100			100			99	100	99	100	100	100
cM capacity (veh/h)	1397			1464			644	602	927	639	601	861
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	111	176	14	0								
Volume Left	0	1	7	0								
Volume Right	2	0	7	0								
cSH	1397	1464	760	1700								
Volume to Capacity	0.00	0.00	0.02	0.00								
Queue Length 95th (m)	0.0	0.0	0.4	0.0								
Control Delay (s)	0.0	0.0	9.8	0.0								
Lane LOS		A	A	A								
Approach Delay (s)	0.0	0.0	9.8	0.0								
Approach LOS			A	A								
Intersection Summary												
Average Delay			0.5									
Intersection Capacity Utilization			19.9%	ICU Level of Service				A				
Analysis Period (min)			15									

HCM Unsignalized Intersection Capacity Analysis

3: Site Access/OPP Driveway & Ontario Street

2028 Future Total PM
02-27-2024

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	0	149	7	7	148	0	5	0	5	0	0	0
Future Volume (Veh/h)	0	149	7	7	148	0	5	0	5	0	0	0
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87
Hourly flow rate (vph)	0	171	8	8	170	0	6	0	6	0	0	0
Pedestrians								19			7	
Lane Width (m)								3.6			3.6	
Walking Speed (m/s)								1.2			1.2	
Percent Blockage								2			1	
Right turn flare (veh)												
Median type		None			None							
Median storage veh												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	177			198			380	387	194	374	391	177
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	177			198			380	387	194	374	391	177
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	100			99			99	100	99	100	100	100
cM capacity (veh/h)	1403			1365			560	536	839	567	533	866
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	179	178	12	0								
Volume Left	0	8	6	0								
Volume Right	8	0	6	0								
cSH	1403	1365	672	1700								
Volume to Capacity	0.00	0.01	0.02	0.00								
Queue Length 95th (m)	0.0	0.1	0.4	0.0								
Control Delay (s)	0.0	0.4	10.5	0.0								
Lane LOS		A	B	A								
Approach Delay (s)	0.0	0.4	10.5	0.0								
Approach LOS			B	A								
Intersection Summary												
Average Delay			0.5									
Intersection Capacity Utilization			23.5%	ICU Level of Service					A			
Analysis Period (min)			15									

HCM Unsignalized Intersection Capacity Analysis

3: Site Access/OPP Driveway & Ontario Street

2033 Future Total AM
02-27-2024

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	0	108	2	1	172	0	5	0	5	0	0	0
Future Volume (Veh/h)	0	108	2	1	172	0	5	0	5	0	0	0
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89
Hourly flow rate (vph)	0	121	2	1	193	0	6	0	6	0	0	0
Pedestrians								11			7	
Lane Width (m)								3.6			3.6	
Walking Speed (m/s)								1.2			1.2	
Percent Blockage								1			1	
Right turn flare (veh)												
Median type		None			None							
Median storage (veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	200			134			328	335	133	330	336	200
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	200			134			328	335	133	330	336	200
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	100			100			99	100	99	100	100	100
cM capacity (veh/h)	1376			1450			616	579	913	612	579	841
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	123	194	12	0								
Volume Left	0	1	6	0								
Volume Right	2	0	6	0								
cSH	1376	1450	736	1700								
Volume to Capacity	0.00	0.00	0.02	0.00								
Queue Length 95th (m)	0.0	0.0	0.4	0.0								
Control Delay (s)	0.0	0.0	10.0	0.0								
Lane LOS		A	A	A								
Approach Delay (s)	0.0	0.0	10.0	0.0								
Approach LOS			A	A								
Intersection Summary												
Average Delay			0.4									
Intersection Capacity Utilization			20.6%		ICU Level of Service		A					
Analysis Period (min)			15									

HCM Unsignalized Intersection Capacity Analysis

3: Site Access/OPP Driveway & Ontario Street

2033 Future Total PM
02-27-2024

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	0	165	7	7	164	0	5	0	5	0	0	0
Future Volume (Veh/h)	0	165	7	7	164	0	5	0	5	0	0	0
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87
Hourly flow rate (vph)	0	190	8	8	189	0	6	0	6	0	0	0
Pedestrians								19			7	
Lane Width (m)								3.6			3.6	
Walking Speed (m/s)								1.2			1.2	
Percent Blockage								2			1	
Right turn flare (veh)												
Median type		None			None							
Median storage (veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	196			217			418	425	213	412	429	196
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	196			217			418	425	213	412	429	196
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	100			99			99	100	99	100	100	100
cM capacity (veh/h)	1381			1343			529	510	819	535	507	845
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	198	197	12	0								
Volume Left	0	8	6	0								
Volume Right	8	0	6	0								
cSH	1381	1343	643	1700								
Volume to Capacity	0.00	0.01	0.02	0.00								
Queue Length 95th (m)	0.0	0.1	0.5	0.0								
Control Delay (s)	0.0	0.4	10.7	0.0								
Lane LOS		A	B	A								
Approach Delay (s)	0.0	0.4	10.7	0.0								
Approach LOS			B	A								
Intersection Summary												
Average Delay			0.5									
Intersection Capacity Utilization			24.3%	ICU Level of Service					A			
Analysis Period (min)			15									

Appendix G

Transportation Tomorrow Survey Data

Cross Tabulation Query Form - Trip - 2016 v1.1

Row: 2006 GTA zone of household - gta06_hhld

Column: 2006 GTA zone of destination - gta06_dest

Filters:

2006 GTA zone of household - gta06_hhld In 8605

Trip 2016

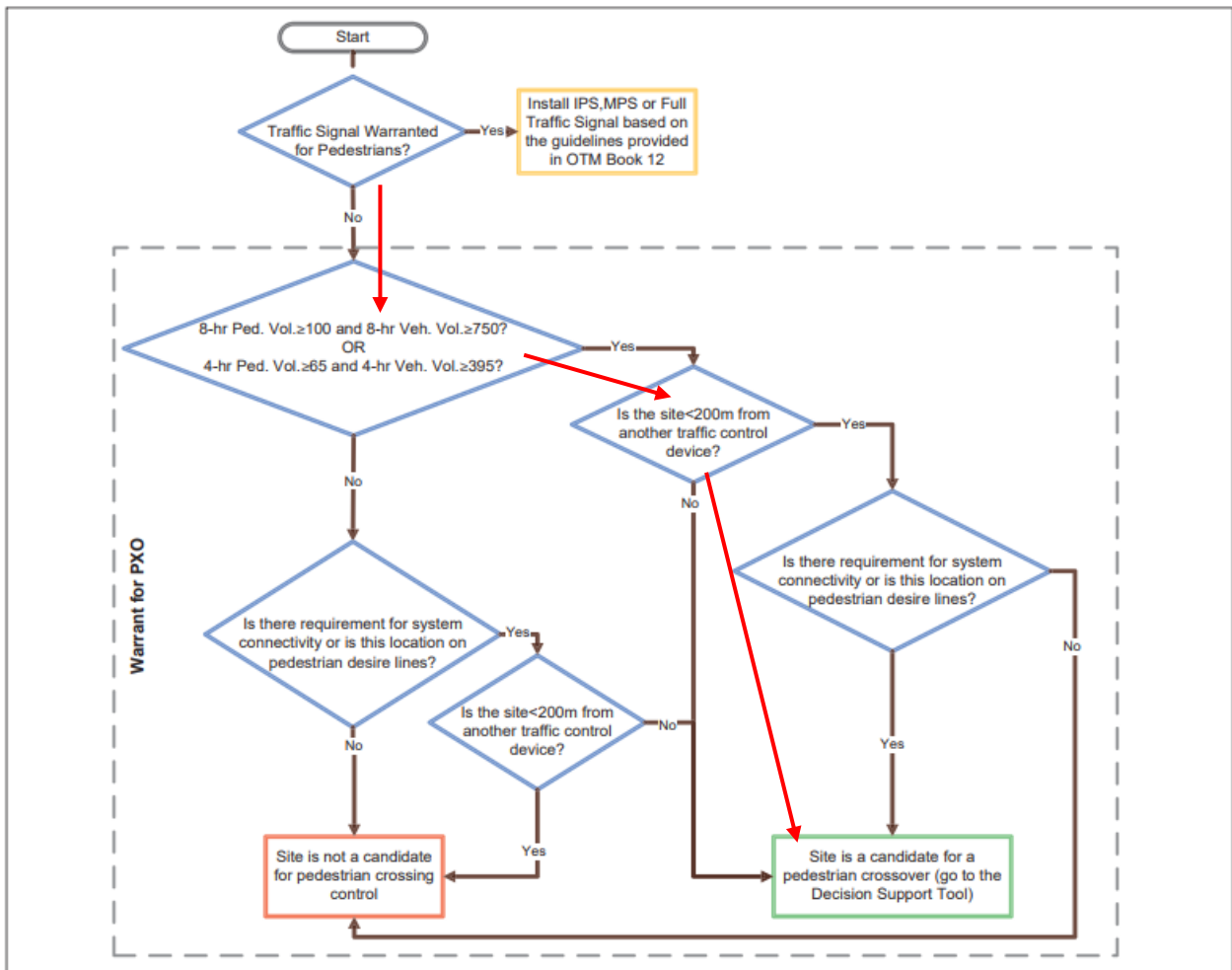
ROW : gta06_hhld

COLUMN : gta06_dest

gta06_hhld	gta06_dest	total		
8605	1132	15	Durham	East 49.21%
8605	1217	15	Durham	West 50.79%
8605	2085	19	York	
8605	2271	77	York	
8605	2606	10	York	
8605	2621	21	York	
8605	3495	21	Peel	
8605	3816	10	Peel	
8605	7125	22	Waterloo	
8605	8121	29	Guelph	
8605	8415	30	Dufferin	
8605	8510	22	Barrie	
8605	8514	8	Barrie	
8605	8526	30	Barrie	
8605	8529	15	Barrie	
8605	8555	17	Simcoe	East
8605	8558	57	Simcoe	East
8605	8567	256	Simcoe	West
8605	8568	258	Simcoe	East
8605	8569	27	Simcoe	East
8605	8605	2289	Simcoe	East/West (Internal)
8605	8607	94	Simcoe	West
8605	8608	318	Simcoe	West
8605	8614	41	Simcoe	East
8605	8617	14	Simcoe	East
8605	8618	23	Simcoe	East/West (South)
8605	8624	32	Simcoe	East
8605	8664	29	Simcoe	East
8605	8714	13	Kawartha	
8605	9032	14	External	omitted
8605	9053	224	External	omitted
8605	9998	69	Undefined	omitted

Appendix H

Ontario Traffic Manual Book 15 Excerpts



Existing Volumes: Wednesday January 31, 2024

Eastbound peak 8 hour count: 725

Westbound peak 8 hour count: 872

Threshold - 8 hour vehicle volumes > 750

Eastbound peak 4 hour count: 457

Westbound peak 4 hour count: 497

Threshold - 4 hour vehicle volumes > 395

Table 7: Pedestrian Crossover Selection Matrix

Two-way Vehicular Volume			Posted Speed Limit (km/h)	Total Number of Lanes for the Roadway Cross Section ¹			
Time Period	Lower Bound	Upper Bound		1 or 2 Lanes	3 lanes	4 lanes w/raised refuge	4 lanes w/o raised refuge
8 Hour	750	2,250	≤50	Level 2 Type D	Level 2 Type C ³	Level 2 Type D ²	Level 2 Type B
4 Hour	395	1,185					
8 Hour	750	2,250	60	Level 2 Type C	Level 2 Type B	Level 2 Type C ²	Level 2 Type B
4 Hour	395	1,185					
8 Hour	2,250	4,500	≤50	Level 2 Type D	Level 2 Type B	Level 2 Type D ²	Level 2 Type B
4 Hour	1,185	2,370					
8 Hour	2,250	4,500	60	Level 2 Type C	Level 2 Type B	Level 2 Type C ²	Level 2 Type B
4 Hour	1,185	2,370					
8 Hour	4,500	6,000	≤50	Level 2 Type C	Level 2 Type B	Level 2 Type C ²	Level 2 Type B
4 Hour	2,370	3,155					
8 Hour	4,500	6,000	60	Level 2 Type B	Level 2 Type B	Level 2 Type C ²	Level 2 Type B
4 Hour	2,370	3,155					
8 Hour	6,000	7,500	≤50	Level 2 Type B	Level 2 Type B	Level 2 Type C ²	Level 1 Type A
4 Hour	3,155	3,950					
8 Hour	6,000	7,500	60	Level 2 Type B	Level 2 Type B		
4 Hour	3,155	3,950					
8 Hour	7,500	17,500	≤50	Level 2 Type B	Level 2 Type B		
4 Hour	3,950	9,215					
8 Hour	7,500	17,500	60	Level 2 Type B			
4 Hour	3,950	9,215					

Type A
 Type B
 Type C
 Type D

Approaches to roundabouts should be considered a separate roadways.

¹The total number of lanes is representative of crossing distance. The width of these lanes is assumed to be between 3.0 m and 3.75 m according to MTO Geometric Design Standards for Ontario Highways (Chapter D.2). A cross sectional feature (e.g. bike lane or on-street parking) may extend the average crossing distance beyond this range of lane widths.

²Use of two sets of side mounted signs for each direction (one on the right side and one on the median)

³Use Level 2 Type B PXO up to 3 lanes total, cross section one-way.

The hatched cells in this table show that a PXO is not recommended for sites with these traffic and geometric conditions. Generally a traffic signal is warranted for such conditions.

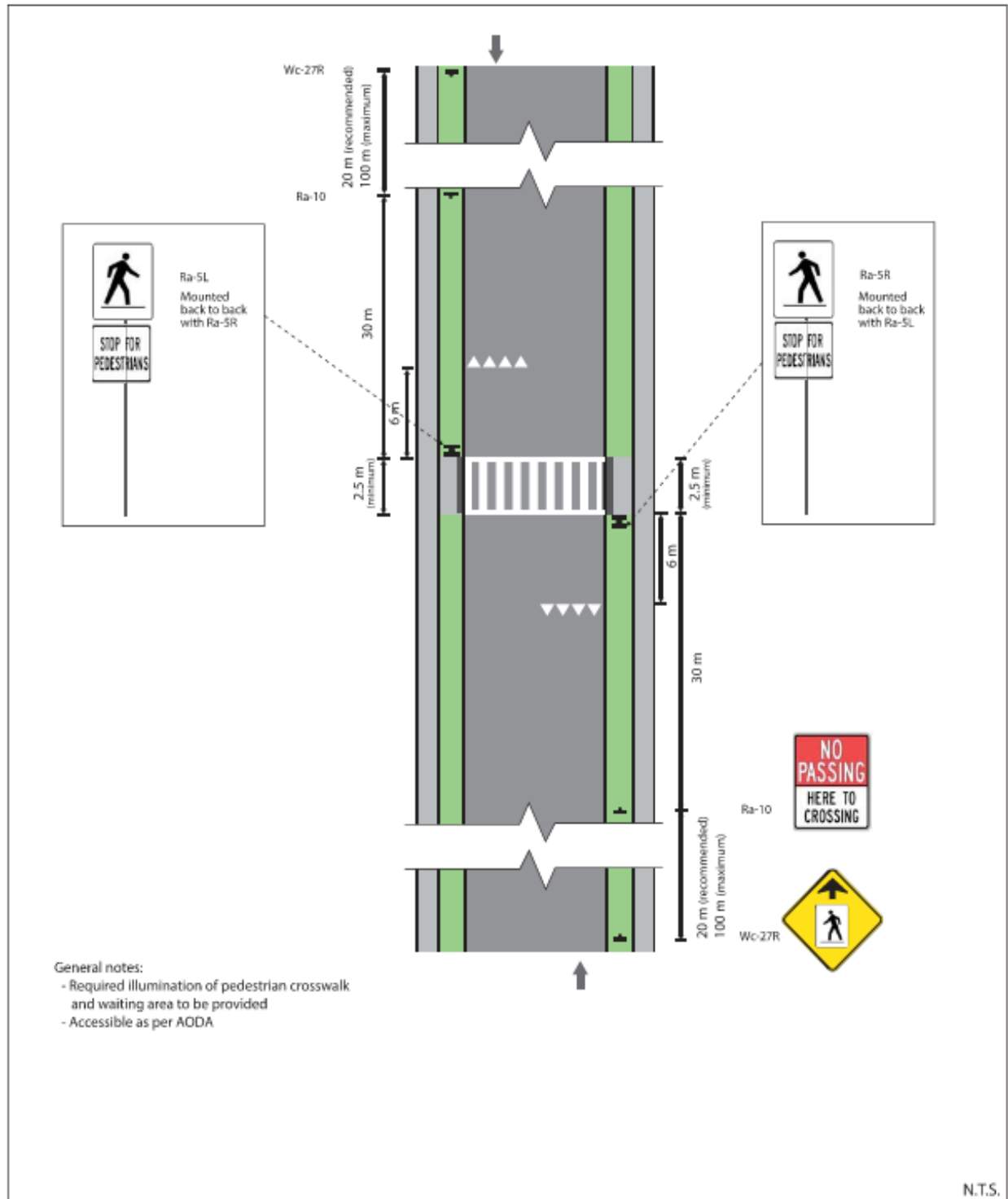


Figure 39: Pedestrian Crossover Level 2 Type D – Mid-block (2-lane, 2-way)

Appendix I

Town of Collingwood Traffic Calming Policy Excerpts



Enhancing our communities



Traffic Calming

POLICIES & PROCEDURES

Town of Collingwood

Document Control

File:

120131

Date:

June
8, 2021

Prepared by:

Tatham Engineering Limited
115 Sandford Fleming Drive, Suite 200
Collingwood, Ontario L9Y 5A6
T 705-444-2565
tathameng.com

Prepared for:

Town of Collingwood
545 Tenth Line
Collingwood, Ontario L9Y 3Z5

Authored by:	Reviewed by:
	 
David Perks, M.Sc., PTP Transportation Planner, Project Manager	Michael Cullip, B.Eng. & Mgmt., P.Eng. Vice President Head Office Operations

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Issue	Date	Description
1	June 8, 2021	Final Policy as Approved by Council

3 Traffic Calming Measures

Traffic calming measures are typically classified into the follow categories:

- physical measures which relate to physical alterations to the road surface or geometry; or
- non-physical measure which relate to “soft” methods such as enforcement, pavement markings and signage.

3.1 PHYSICAL MEASURES

Physical measures include:

- speed humps (rounded raised areas placed across the roadway);
- speed cushions (variation of speed humps with gaps in the humps to accommodate emergency vehicles and busses);
- speed tables (flat-topped speed humps);
- rumble strips (raised bars/grooves closely spaced);
- textured crosswalks (patterned surface contrasting the adjacent roadway);
- raised intersections (flat raised areas covering entire intersections, with ramps on all approaches and often with brick or other textured materials on the flat section);
- traffic circles (raised island, placed in intersections, around which traffic circulates);
- roundabouts (larger than traffic circles and typically have raised splitter islands to channel approaching traffic to the right and are used on higher volume streets);
- chicanes (curb extensions that alternate from one side of the street to the other, forming S-shaped curves);
- chokers (curb extensions at midblock locations that narrow a street);
- curb radius reductions (reconstruction of an intersection corner to a smaller radius);
- realigned intersections (changes in alignment that convert T-intersections with straight approaches into curving streets that meet at right angles);
- neckdowns (curb extensions at intersections that reduce roadway width curb to curb);
- centre island narrowings (placement of a raised island located along the centerline of a street that narrows the travel lanes at that location);
- on-street parking (allowing vehicles to park adjacent and parallel to the curb); and



- gateways (a combination of traffic calming measures that identify a transition between rural and urban zones - ideal for arterial roads).

3.2 NON-PHYSICAL MEASURES

As previously noted, non-physical traffic calming measures are usually implemented through enforcement, signage and pavement markings. Such measures include:

- speed enforcement (police enforcement, particularly in school and community safety zones; can also include automated speed enforcement);
- pavement marking legends (painted speed limits on the pavement surface to remind drivers of the speed limits);
- transverse lane markings (transverse bars or chevron pavement markings on a travel lane);
- lane narrowing and shoulder widening through pavement marking; and
- dynamic speed displays (radar signs which indicate travel speeds).

It is noted that while stop signs are often employed as a means of traffic calming, such is not recommended. As per the *Ontario Traffic Manual Book 5 Regulatory Signs*, stop signs should not be used for speed control. Unwarranted stop signs increase vehicular speeds between stop signs (stop signs only affect speeds within approximately 40 metres of the stop sign), encourage rolling stops and ultimately, non-compliance. An excessive number of stop signs, particularly those that are not warranted, also breeds disrespect for stop signs and other traffic control devices.

3.3 TOWN OF COLLINGWOOD MEASURES

In consideration of the Town objectives in implementing traffic calming guidelines, and recognizing the combination of urban, semi-urban and rural roads within the Town's road network, the following traffic calming measures have been considered:

- | | | |
|-------------------------------------|---------------------------|------------------------|
| ▪ police enforcement; | ▪ chicanes; | ▪ speed cushions; |
| ▪ automated speed enforcement; | ▪ curb radius reductions; | ▪ speed tables; |
| | ▪ curb extensions; | ▪ centre median; |
| ▪ dynamic speed signs; | ▪ traffic calming curbs; | ▪ traffic circles; and |
| ▪ lane narrowing via road markings; | ▪ textured cross walks; | ▪ on-street parking. |
| | ▪ speed humps; | |



Dynamic Speed Signs

Dynamic speed signs are portable or permanent radar activated signs that instantaneously display approaching speeds for individual vehicles. They can also be programmed to display appropriate messages (e.g. Too Fast or Slow Down). These devices create a sense of being monitored to the driver and provide an instant notification that the speed limit is being exceeded (if such is the case).

It is recommended that the Town require implementation of dynamic speed signs at sensitive locations (i.e. parks, school zones, school crossings, collector streets, etc.) as part of the approval process for greenfield subdivisions.



Permanent installations of dynamic speed signs can lose their effectiveness over time as drivers become accustomed to them and presence and message gets ignored. Notwithstanding, application in unique circumstances (at schools or near parks) is considered beneficial. The use of portable signs would allow the Town to position signs at varying locations throughout the Town on a short-term basis, thus providing the appropriate messaging to motorists with the intent of changing their driving behaviour.

Advantages

- Educational tool, good public relations, effective as a temporary speed reduction measure.
- Can help residents understand the actual speed of a vehicle vs perceived speed.
- No impact to emergency vehicle operations or road maintenance operations.

Disadvantages

- Relies on motorist to voluntarily comply, duration of effectiveness is limited, not accurate on two-lane roads (too much traffic).
- Requires regular maintenance and a source of power.
- Light pollution on residential streets.

Estimated Cost

- \$8,000 for sign.
- \$25,000-\$35,000 for trailer.



Textured Crosswalks

Textured crosswalks incorporate a textured and/or patterned surface which contrasts the adjacent roadway. Textured crosswalks are utilized to enhance visual identification of a traffic calmed area, emphasizing pedestrian priority. Priority locations include high pedestrian crossing locations (i.e. in downtown area, park and school locations as well as trail crossings).



Advantages

- Improves pedestrian safety.
- No impacts to emergency services or snow plowing operations.
- Enhanced roadway aesthetics.

Disadvantages

- May create additional noise.
- Traction or stability problems for seniors, disabled individuals, wheelchairs etc.
- Increased maintenance depending on base stability (maintenance to repair uneven transition between textured crosswalk and asphalt).

Estimated Cost

- \$50 to \$150 per m².



Speed Humps

Speed humps are defined as a raised area of the road which deflects both the wheels and frame of a traversing vehicle. Typically, speed humps are 80 mm high and 4.0 metres wide (in the direction of travel), and spaced 125 to 225 metres apart. Speed humps are used on residential streets and, in some instances, on collector roads.



Advantages

- Relatively cost-efficient.
- Easy to construct.
- Deters cut-through traffic.
- Reduces vehicle speeds.

Disadvantages

- Increases emergency vehicle response time.
- Possible noise from braking/acceleration.
- Potential impacts to snow plows and trucks.
- Added wear and tear on vehicles over time (including snow plows).

Estimated Cost

- \$2,000 to \$3,000 each.



Speed Cushions

A speed cushion is a variation on a speed hump that can be straddled by large vehicles such as emergency vehicles and busses, but passenger vehicles are impacted in the same manner as a speed hump.



Advantages

- Relatively cost-efficient.
- Easy to construct.
- Deters cut-through traffic.
- Reduces vehicle speeds.
- Does not disrupt speed of emergency or transit vehicles.

Disadvantages

- Possible noise from braking/acceleration.
- Potential impacts to snow plows and trucks.
- Drivers may try to straddle the cushions on right side of road and impact bicycle lanes.
- Added wear and tear on vehicles over time.

Estimated Cost

- \$4,000 for a set of 3.



Speed Tables

Speed tables are flat-topped asphalt or rubber mounds that cover the full width of the roadway. The ramps of the speed table are more gently sloped than speed humps and thus speed tables are less jarring than a standard speed hump and can allow larger vehicles (emergency vehicles, trucks, snow plows, etc.) to cross with reduced disruption. Supplementary text can be added to the pavement in advance of the speed table (i.e. SLOW DOWN).



Advantages

- Relatively cost-efficient.
- Easy to construct.
- Deters cut-through traffic.
- Reduces vehicle speeds.
- Lesser impacts to larger vehicles as compared to speed humps.

Disadvantages

- May delay emergency vehicle response times.
- Traffic may divert traffic to alternate routes.
- Possible noise created by braking/acceleration.
- Negative impacts to snow plows operations.
- Added wear and tear on vehicles over time.

Estimated Cost

- \$3,000 to \$5,000 each.
- \$10,000 for a modular speed hump.



4 Traffic Calming Policy

4.1 CONSIDERATION FOR TRAFFIC CALMING

Traffic calming measures should:

- be considered only after education and enforcement efforts have failed to produce the desired results;
- be considered when there is a demonstrated safety or speed concern and acceptable alternative measures have been exhausted;
- be considered after focus is placed first on improvements to the arterial road network, such as signal timing optimization (thereby making those roads more appealing);
- be considered in new plans of subdivision where the absence of such measures is likely to result in speeding and/or related safety concerns;
- be predominantly restricted to two lane roads (one lane of through traffic in each direction);
- maintain reasonable automobile access to Town roads;
- only be installed after Town staff have investigated existing traffic conditions and the necessary approvals have been received; and
- be monitored with follow-up studies to assess their effectiveness, with the results communicated to the community and Council.

Traffic calming measures should not:

- impede emergency and transit services access unless alternate measures are agreed upon;
or
- impede non-motorized, alternative modes of transportation such as walking and cycling.

4.2 GREEN FIELD DEVELOPMENT

It is noted that the traffic calming policy and associated process contained herein are intended to inform the implementation of traffic calming measures in existing areas or developments where conditions on the surrounding road network have evolved over time to the point that the intended function of the road network has been compromised. With respect to green field development, a traffic calming review should be conducted during draft plan design. The potential need for traffic calming measures will be discussed during pre-consultation, with the intent that a traffic calming plan (should such be deemed necessary) be submitted for review by



the Town. Upon review and assessment of the traffic calming plan, any traffic calming measures to be implemented will be added to the draft plan conditions for the respective development.

The traffic calming measures identified in Section 3.3 may also be considered for green field developments. Additionally, roundabouts can be considered within new developments, as may be appropriate. Roundabouts provide similar advantages to traffic circles; however, they are typically installed at higher volume intersections. Given the additional right-of-way typically required to construct a roundabout, the implementation of such is ideal in green field developments (i.e. where there are no constraints imposed by existing development).

4.3 COMMUNITY INVOLVEMENT

Restoring neighbourhood streets to their intended function and improving overall livability are the primary objectives of traffic calming. In order to achieve this goal, community involvement and support is paramount.

Communication with residents should occur at various stages throughout the process as the traffic calming plan is developed and implemented. Traffic calming plans should be developed with an understanding of current and historical traffic patterns within the area under investigation. For a traffic calming program to be successful, the community must support and be committed to the solution. The only means of gaining this commitment is to involve the residents by informing them of the study location being considered for traffic calming measures.

The benefit of community involvement is that it generates support for a traffic calming program and assists in the implementation of a plan without significant opposition upon completion. Community involvement also enhances the credibility of the traffic calming program, particularly when it is ultimately presented to Council for approval.

The review and implementation of traffic calming measures is a time consuming and expensive process requiring many resources. Without public support, the traffic calming measures intended to alleviate traffic concerns could be met with negative public opinion, potentially jeopardizing the outcome and positive impacts to affected neighbourhoods.

Neighbourhood support, enforcement, education of motorists, bicyclists and pedestrians, appropriate engineering applications and economics typically determine the success of any traffic calming endeavor. A cooperative partnership between the affected residents and the Town is essential to the success of the project.

4.4 CLASS ENVIRONMENTAL ASSESSMENT PROCESS

Traffic calming is currently exempt from the Ontario Environmental Assessment Act and is not an undertaking subject to the *Municipal Engineers Association Municipal Class Environmental*



Assessment (October 2000, as amended in 2007, 2011 & 2015). Under the proposed 2020 MEA Class EA Amendments, the implementation of traffic calming measures is to be a Schedule A+ undertaking. Schedule A+ projects are pre-approved, provided that the public is advised prior to implementation.



5 Traffic Calming Process

The following process will be used when proceeding with a request for traffic calming measures within the Town of Collingwood (corresponding flow chart is provided in Appendix A). An established and formal process for investigating roads provides consistency and equality in the determination of need and suitability of traffic calming measures.

5.1 STEP 1: TRAFFIC CALMING REQUEST

Residents with traffic related concerns are instructed to submit their written request, accompanied by a petition, to the Town to investigate traffic calming on their road or within their neighbourhood. Town staff will provide a copy of the petition to the proponent (refer to Appendix B). The purpose of the petition is to establish whether or not there is sufficient neighbourhood/local support for traffic calming measures prior to the Town initiating an investigation into the need for such measures on the subject road. The petition serves as a preliminary screening tool that will prevent unsupported traffic calming requests from being advanced.

The petition results must clearly demonstrate that a minimum of 51% of the dwelling units with direct frontage or flankage onto the candidate road or road section support the potential implementation of traffic calming measures, as defined by Town staff. Each dwelling unit is represented by one signature, regardless of the number of people in the unit. Failure to satisfy the minimum support threshold of 51% will result in termination of the investigation. Successful petitions (i.e. those that satisfy the required 51% support threshold) will trigger the screening by Town staff (Step 2).

5.2 STEP 2: TOWN SCREENING

Following an appropriate request for traffic calming consideration, Town staff will undertake a screening of the request and candidate road section considering road classification, traffic volumes, length, grade and actual travel speeds.

Initial Criteria

Initial screening criteria to determine eligibility for consideration for traffic calming measures have been established. With respect to the road or road section in question, it must:

- be a local road assumed and maintained by the Town of Collingwood;
- have a minimum annual average daily traffic (AADT) volume of 900 vehicles;



- have a minimum uncontrolled (i.e. no stop signs or traffic signals) length of 220 metres without being a dead-end road section or cul-de-sac;
- have a grade that does not exceed 6%; and
- not have been the subject of a previous speed study or traffic calming request within the past 5 years.

In addition, the following must also be satisfied:

- all reasonable efforts must have been made to address the concerns utilizing other means including education and enforcement tools; and
- zoning should be primarily residential in nature.

If the subject road or road section does not satisfy the above criteria, it will not be considered an appropriate candidate for traffic calming.

While the focus of traffic calming will be on local roads, Town staff may, at their discretion, review select collector roads for consideration provided that they also meet the above criteria and serve in excess of 2000 vehicles per day.

Speed Criteria

For locations meeting the initial screening criteria, a travel speed survey will be conducted to determine whether speeding is occurring through the study area. For vehicle speeds, it is not prudent to consider the highest speed at which motorists travel. Rather, the 85th percentile speed is typically considered, which is the speed at which 85% of the total traffic volume on a road is travelling at or below. The 85th percentile concept is based on the theory that the large majority of drivers:

- are reasonable and prudent;
- do not want to be involved in a motor vehicle accident; and
- desire to reach their destination in the shortest possible time.

Based on these assumptions, the 85th percentile speed (which represents the large majority) observed under good conditions (i.e. favourable weather and visibility) may be considered as the maximum safe speed for that location. The speed limit and 85th percentile speed should be relatively comparable - thus indicating that the function and physical characteristics of the road are properly communicated, understood and respected by motorists. Ideally, the 85th percentile speed should be ± 10 km/h of the speed limit. Where the 85th percentile speed exceeds the posted speed by more than 10 km/h, it is an indication that intervention is required to reduce vehicle operating speeds. A lower tolerance may be applied in reduced speed zones and



community safety zones where stricter adherence to the speed limit is desired for obvious reasons.

In considering the need for traffic calming, the 85th percentile speed must exceed the posted speed limit by the values provided in Table 2.

Table 2: 85th Percentile Speed Considerations

POSTED SPEED LIMIT	85 TH PERCENTILE SPEED	EXCEEDANCE OF SPEED LIMIT
40 km/h	45 km/h	+5 km/h
50	60	+10
60	70	+10

If the observed 85th percentile travel speed exceeds the posted speed limit by the threshold noted in Table 2, the road section will be identified as a candidate for traffic calming measures and will proceed to Step 3. Road sections with 85th percentile travel speeds that do not exceed the noted thresholds will be disqualified from consideration.

Resident Notification

Following the screening process, the Town will inform the resident(s) in writing as to whether their location meets the screening criteria for implementation of traffic calming measures. For locations not meeting the screening criteria, investigation for traffic calming will cease and the location will be precluded from future traffic calming consideration for a period of 5 years.

5.3 STEP 3: DATA COLLECTION

If the requested location satisfies the petition requirements and screening criteria, additional data collection and analysis will commence. The collection of traffic data, as deemed necessary by Town staff, will serve to provide a better understanding of the current traffic conditions, inform the selection of appropriate traffic calming measures and prioritize locations for the implementation of traffic calming.

Staff will conduct/coordinate the necessary traffic studies to quantify and qualify the submitted traffic concerns. The data collected may include traffic volumes and composition (cars and trucks), collision records (for those that are attributed to excessive speeding), pedestrian activity and historical site-specific information. It is noted that traffic volumes and vehicle composition data can be gathered with the speed surveys conducted in Step 2.



5.4 STEP 4: PRELIMINARY DESIGN & REVIEW

Traffic Calming Measure

To identify potential traffic calming measures, a comprehensive review of the data collected in Step 3 will be conducted, supplemented by site visits, historical information, future maintenance and construction plans, as well as stakeholder feedback. Appropriate traffic calming measures will be determined based on the list of traffic calming measures outlined in Section 3.3. The traffic calming strategy could include one or more of the noted traffic calming techniques. The proposed traffic calming measures will be selected at the sole discretion of the Town, in accordance with the design guidelines outlined in the *Canadian Guide to Traffic Calming* and the judgment and experience of Town staff.

Initial Preliminary Design

Following selection of the traffic calming measure(s) to be implemented, Town staff will prepare preliminary design drawings illustrating the proposed solution.

Agency/Department Review

Staff will provide the preliminary design drawings to the relevant agencies and/or Town departments (e.g. emergency services, transit services, road operations, applicable Town advisory committees, etc.) for review and comment. As required, Town staff will work with the agencies/departments to modify the preliminary design to address concerns. If however, concerns cannot be appropriately mitigated to ensure support for the traffic calming measure, the traffic calming process will be discontinued for the road under investigation.

Final Preliminary Design

Further to the agency/department review and in keeping with the goals, objectives and principles set out in these guidelines, staff will finalize the preliminary traffic calming design. General consideration will be given to the various aspects of road design such as utility placement, landscaping, signage requirements and drainage with the purpose of establishing a cost estimate for implementation.

5.5 STEP 5: COMMUNITY NOTIFICATION

Using summarized comments from the submitted petition and preliminary information about the subject road and surrounding area, staff will define the limits for community notification, which may include surrounding roads and neighbourhoods. As a minimum, dwelling units with direct frontage onto the road to be investigated will be notified via direct mailings and/or email, in addition to each property whose side yard abuts the subject road section. Notifications will



include a description of the intended traffic calming strategy and its location. The noted information will also be posted on the Town website and social media platforms.

5.6 STEP 6: PRIORITIZATION

Prioritization Assessment

The prioritization assessment is a process that weighs and scores various attributes of a candidate road or road section with the intent of quantifying and prioritizing the urgency for traffic calming. A predetermined set of road attributes are assigned weighted points based on the severity of each individual attribute (e.g. 85th percentile speed, pedestrian volumes, etc.). The candidate road is reviewed and scored in context of these attributes. This process quantifies the conditions of the road and further provides a method of prioritizing each candidate location in terms of need. A basis for assessment has been prepared and is provided in Appendix C.

Locations will be prioritized based on the point system, with those locations with the highest points implemented first. If funding does not permit all locations to be implemented in one year, roads will be carried forward to the next year when they will then be re-prioritized to include any new locations. While the intent is to implement traffic calming measures according to the priority ranking, where funding does not permit all pending projects to be implemented, locations with a lower priority rank may be implemented ahead of higher ranking projects at the Town's discretion to ensure use of available funding is maximized.

Council Notification

During yearly budget deliberations, Town staff will propose one or more traffic calming projects to be implemented. Projects put forward will be based on the priority ranking. Where necessary, an amending By-law will be brought to Council for approval (ie. changes to regulatory signage, etc.).

5.7 STEP 7: FINAL DESIGN & IMPLEMENTATION

Final Design

In advance of implementation, detailed engineering drawings will be completed as necessary. These drawings will provide a high level of detail taking into consideration but not limited to the following:

- surface drainage;
- sub-base requirements (i.e. granular type and thickness);
- surface type (asphalt, concrete, decorative concrete);
- road grade;



- requirements for warning signs and pavement markings;
- sightlines and sight distances;
- driveway and intersection locations; and
- utility locations or relocations.

At this point, the feasibility and costing of the preferred traffic calming measures will be evaluated in detail. If, during the detailed design stage, limitations are identified which challenge the feasibility of the plan, alternatives will be considered. This may include alterations or a re-development of the preferred plan. If staff believe that the required modifications to create the detailed design result in a significantly different final design from that which was originally presented, staff may recommend additional agency/department reviews to inform the revision process.

Community Notification

Notices detailing planned project timing will be distributed to residents prior to implementation, in the same manner as the previous notification. The notices will also be posted on the Town website and social media platforms.

Implementation

Following resident notification, traffic calming measures will be implemented. Where feasible, staff may decide it is beneficial to phase in the traffic calming plan through the use of temporary or removable traffic calming measures such as pavement markings, curbs/barriers, planters or barrels. This will allow time to examine the impact of the measures and their effectiveness before implementing permanent treatments.

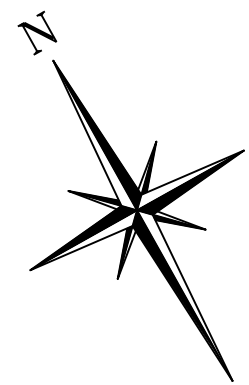
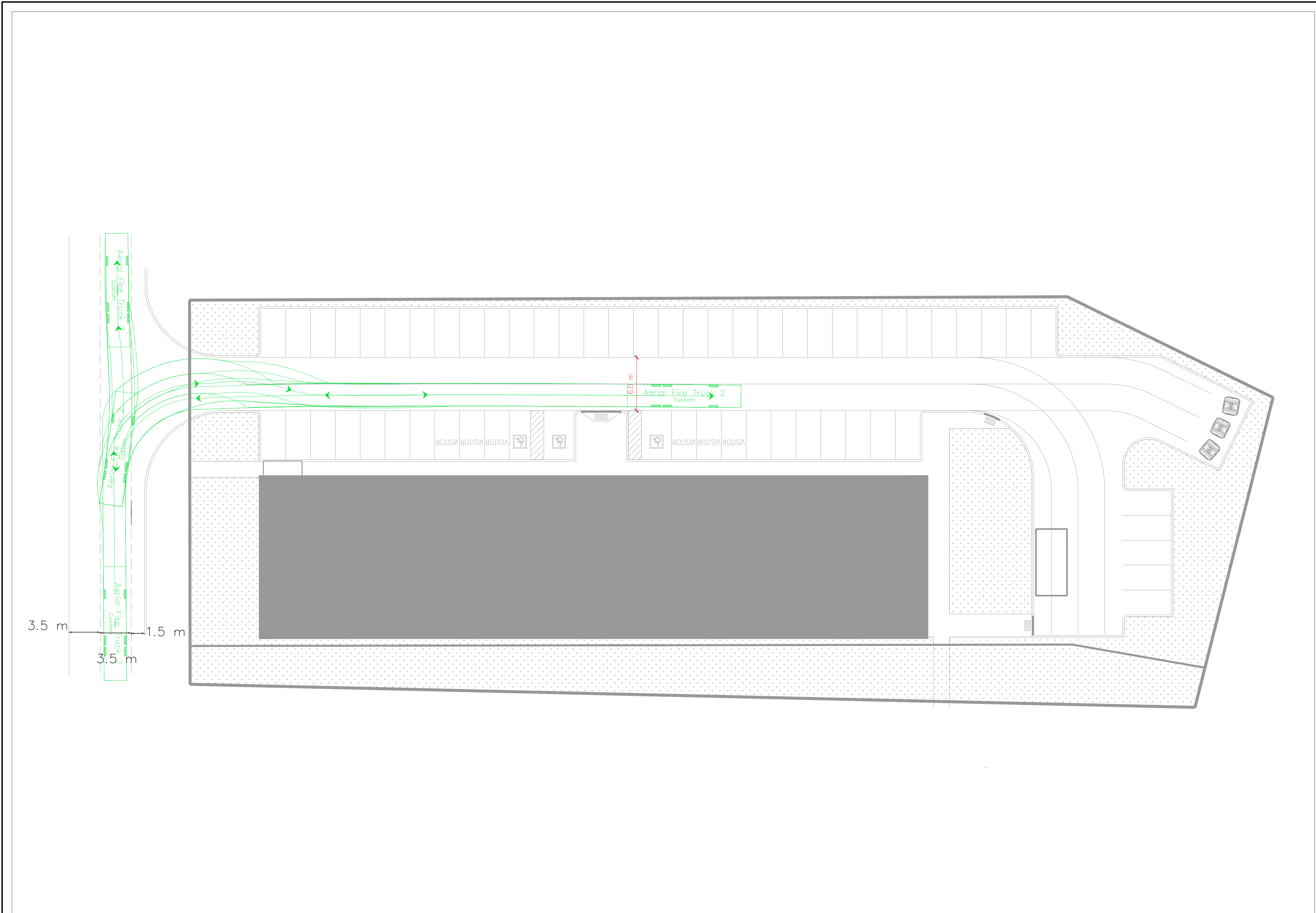
5.8 STEP 8: MONITOR & EVALUATE

Town staff may monitor the road to determine the effectiveness of the utilized measures and their impact on the surrounding road network. This information will be used in recommending similar measures within the Town in the future.

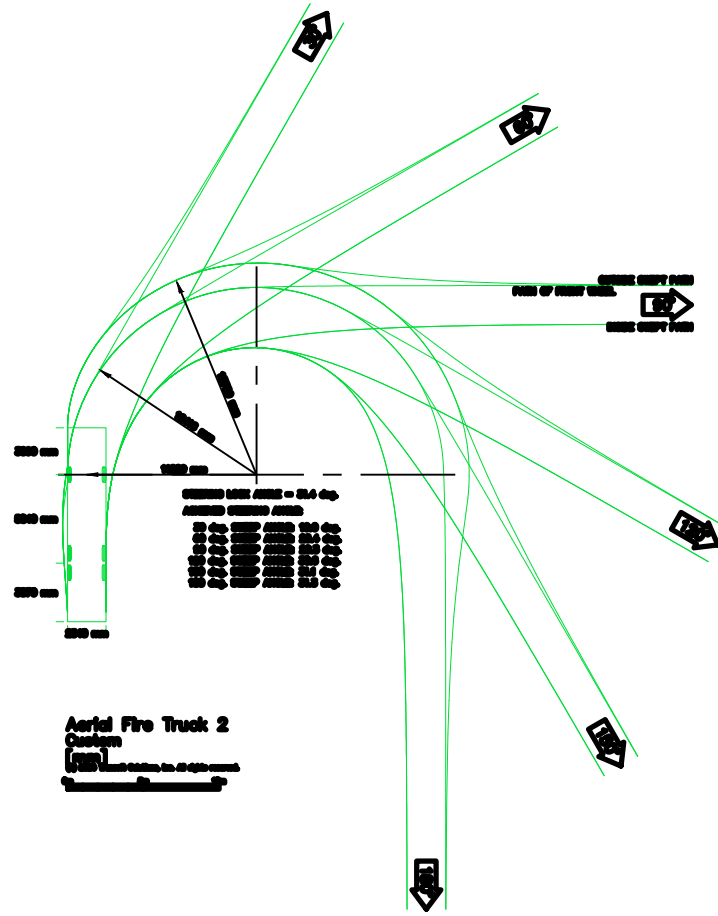


Appendix J

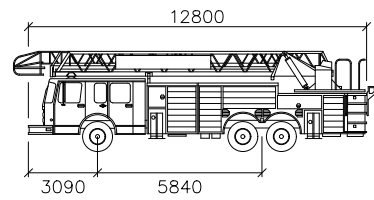
Vehicle Maneuverability Diagram



SWEEP PATH



VEHICLE PROFILE



Aerial Fire Truck 2
mm
Width : 2540
Track : 2540
Lock to Lock Time : 6.0
Steering Angle : 31.4

No.	ISSUE	DATE: MM/DD/YYYY
0	ISSUED FOR REVIEW	02/01/2024

Engineer

PRELIMINARY
NOT TO BE USED FOR CONSTRUCTION

Engineer

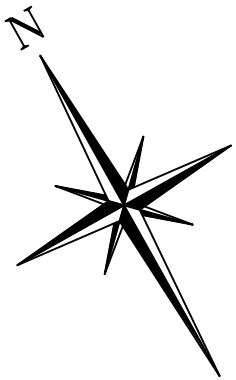
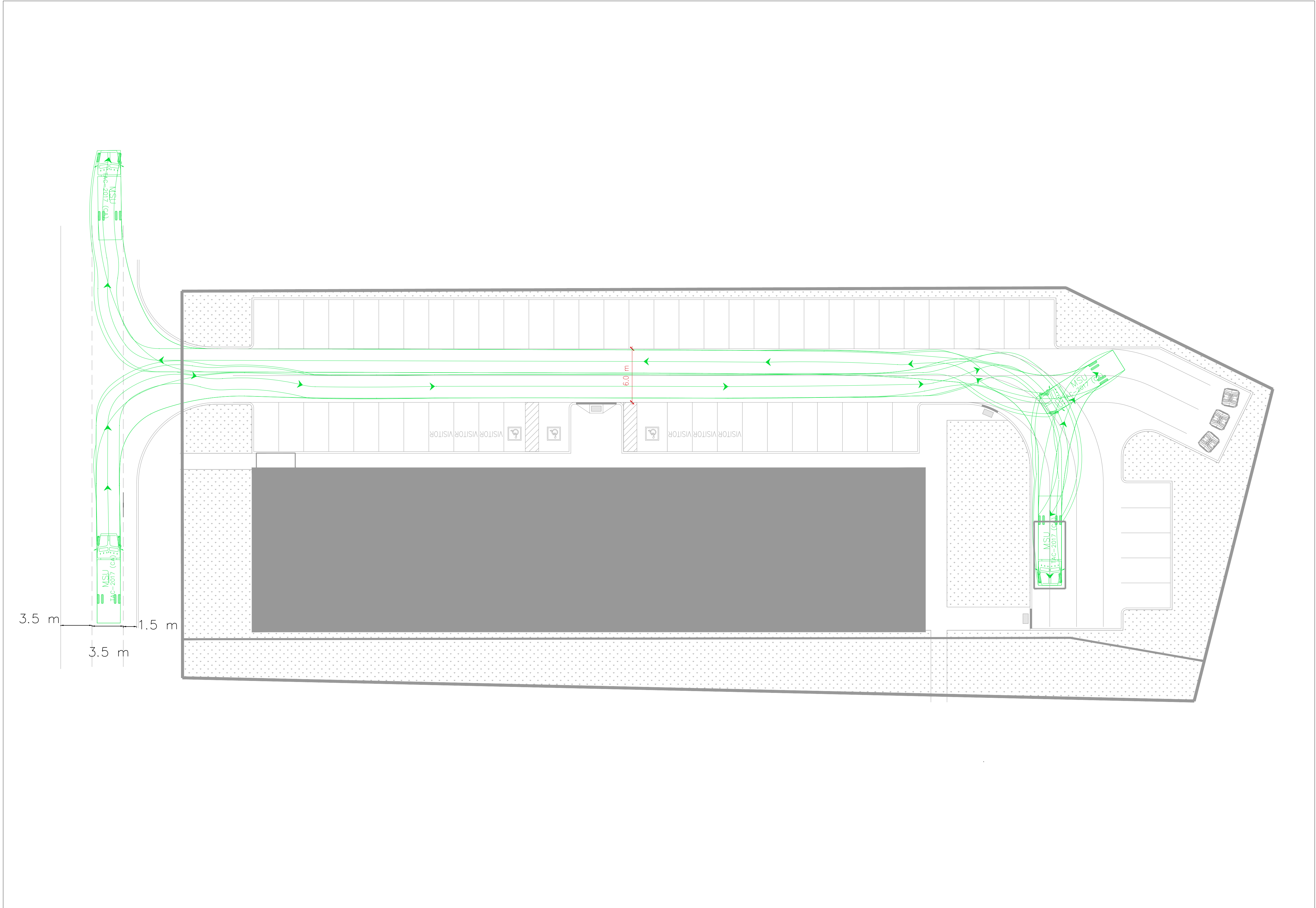
Project
180 ONTARIO ST

Drawing
VEHICLE MANEUVERING DIAGRAM
FIRE TRUCK

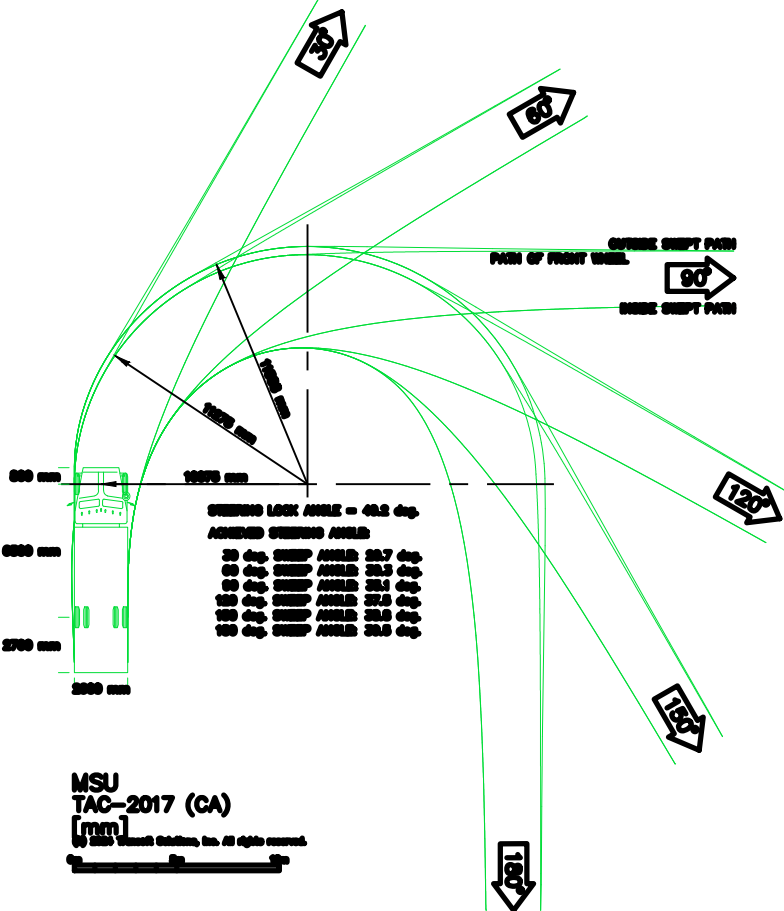


2800 HIGH POINT DRIVE
SUITE 100
MILTON, ON, L9T 6P4
905-875-0026 T
905-875-4915 F
info@crozier.ca
www.crozier.ca

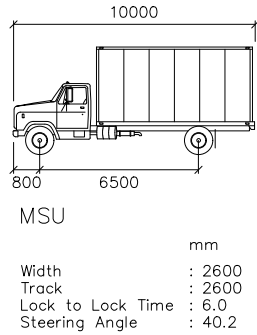
Drawn By B.L.	Design By B.L.	Project 2598-6970
Check By D.B.	Check By M.F.	Scale N.T.S.
		Drawing T300



SWEEP PATH



VEHICLE PROFILE



No.	ISSUE	DATE: MM/DD/YYYY
0	ISSUED FOR REVIEW	02/01/2024

Engineer

Engineer

PRELIMINARY
NOT TO BE USED FOR CONSTRUCTION

Project
180 ONTARIO ST

Drawing
VEHICLE MANEUVERING DIAGRAM
MEDIUM SINGLE UNIT TRUCK

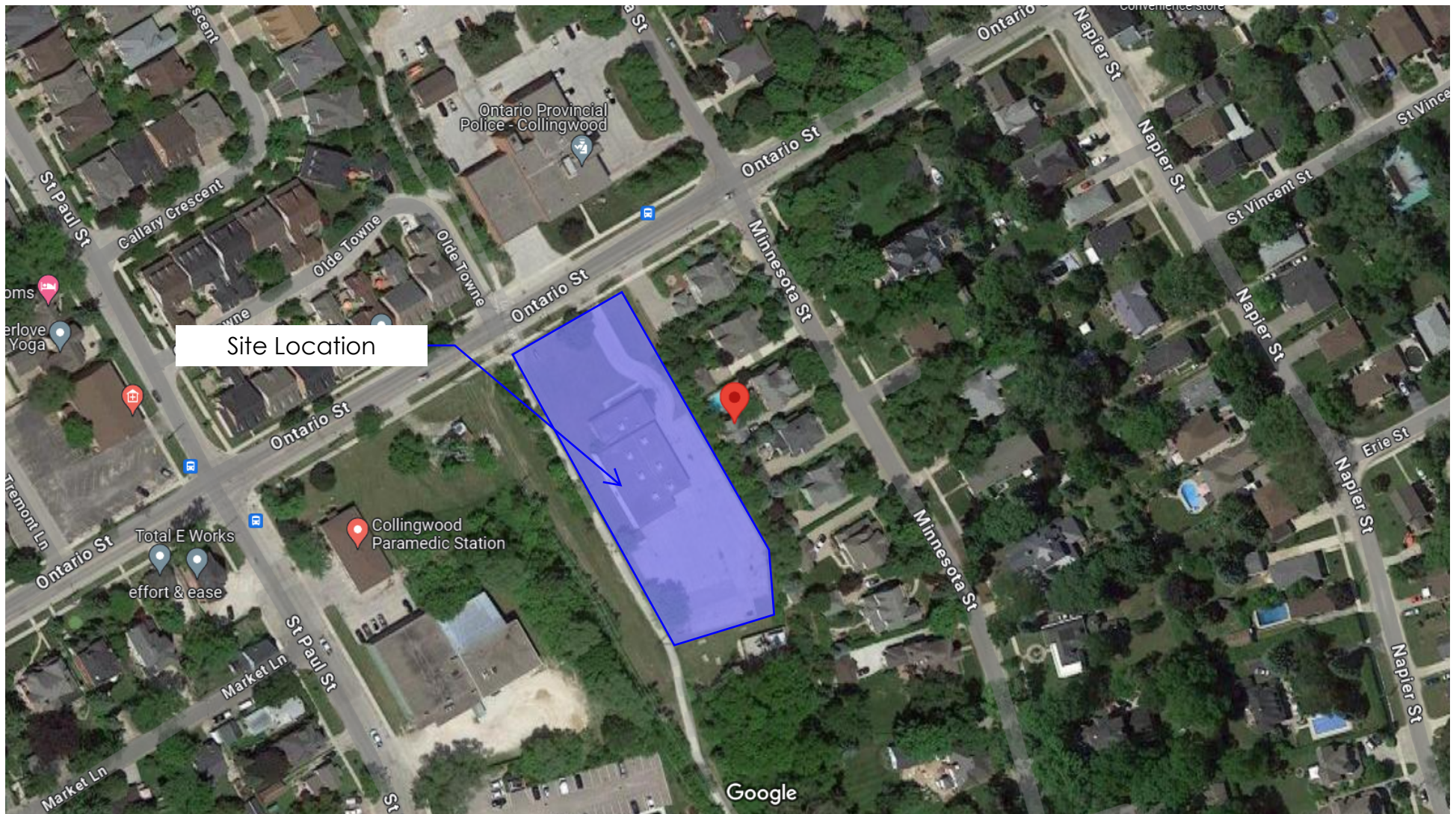


CROZIER
CONSULTING ENGINEERS

2800 HIGH POINT DRIVE
SUITE 100
MILTON, ON L9T 6P4
905-875-0026 T
905-875-4915 F
info@crozier.ca

Drawn By	B.L.	Design By	B.L.	Project	2598-6970
Check By	D.B.	Check By	M.F.	Scale	N.T.S.
				Drawing	T302

FIGURES

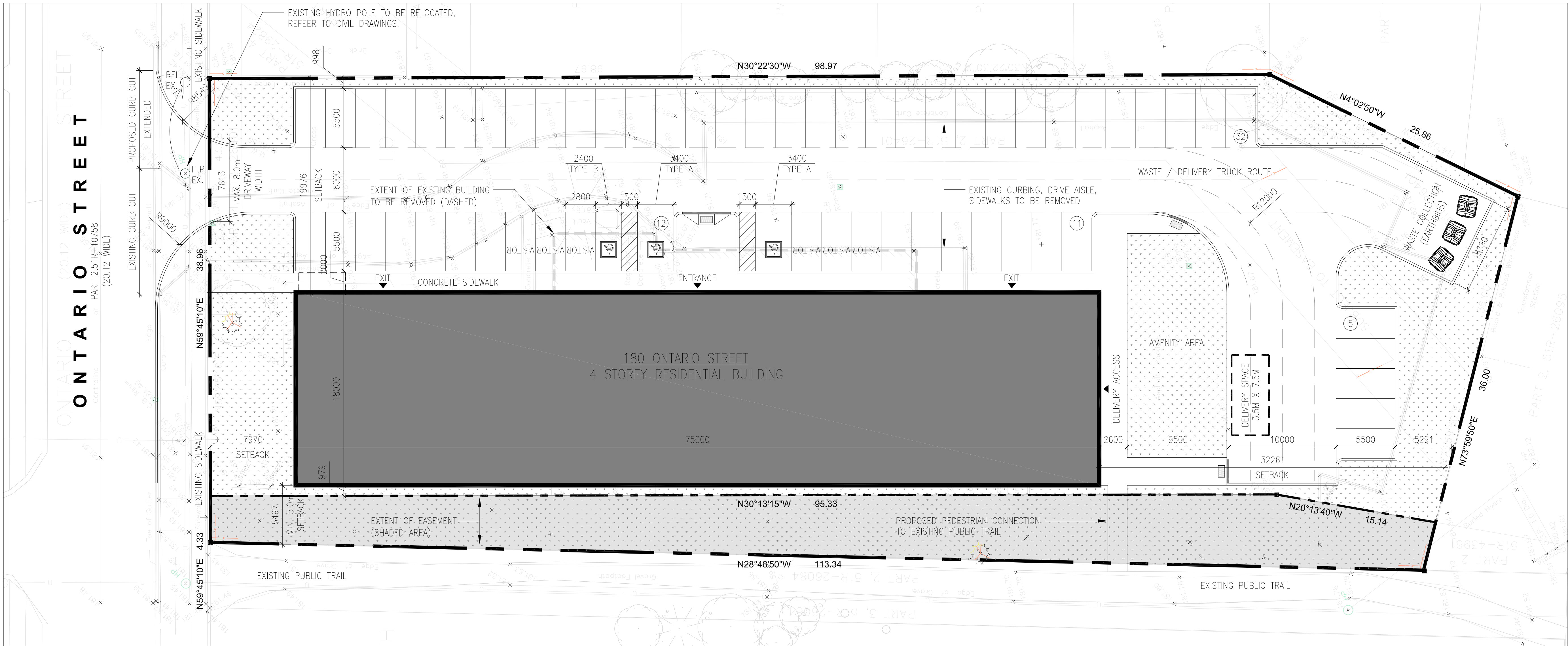


Project	180 ONTARIO STREET	
Drawing	SITE LOCATION PLAN	



ADMIRAL BUILDING
1 FIRST STREET, SUITE 200
COLLINGWOOD, ON L9Y 1A1
705 446-3510 T
705 446-3520 F
WWW.CFCROZIER.CA

Drawn By	D.B.	Design By	D.B.	Project	2598-6970
Scale	N.T.S.	Date	JAN. 17, 2023	Check By	
					Drawing
					FIG. 1

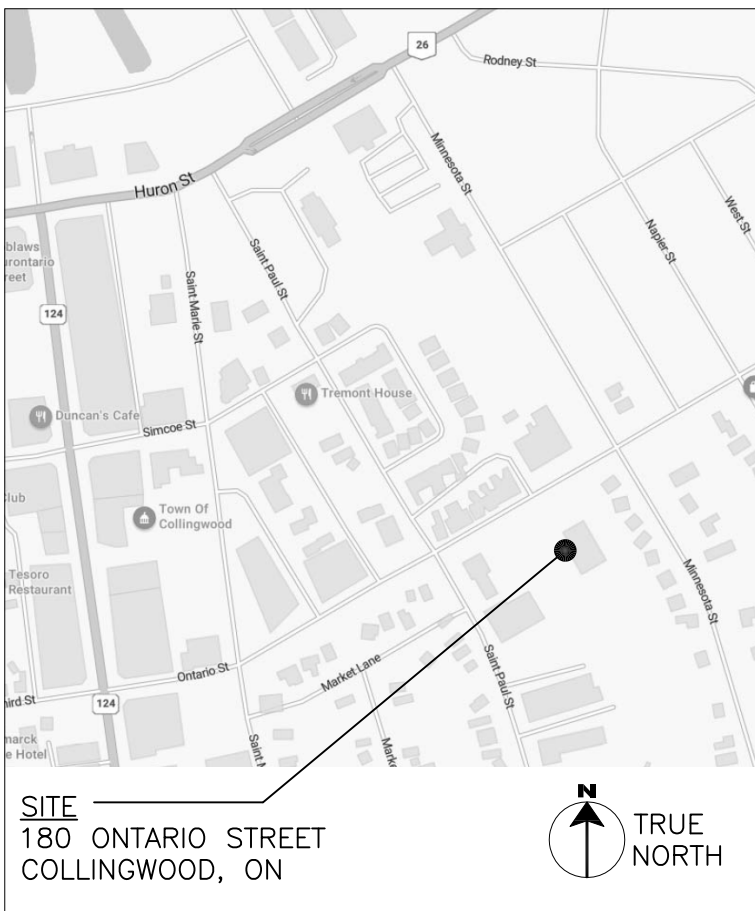


1
A100 PROPOSED SITE PLAN
SCALE: 1:200

ZONING REQUIREMENTS		
ZONING CATEGORY: RESIDENTIAL FOURTH DENSITY (R4) ZONE		
LOT REGULATIONS	REQUIRED	PROVIDED
MINIMUM LOT AREA	NIL	5,193 sqm
MINIMUM LOT FRONTAGE	30.0 m	43.29 m
MINIMUM FRONT YARD	7.5 m	7.97 m
MINIMUM INTERIOR SIDE YARD (WEST)	7.5 m	5.49 m
MINIMUM INTERIOR SIDE YARD (EAST)	7.5 m	19.97 m
MINIMUM REAR YARD	7.5 m	32.26 m
MAXIMUM HEIGHT	18.0 m	18.00 m
MAXIMUM LOT COVERAGE	40 %	26.00 %
MINIMUM LANDSCAPED OPEN SPACE	40 %	(1,529 sqm) 29.44 %
GROSS FLOOR AREA		
TYPICAL FLOOR AREA	1,350 sqm	14,531 sqft
TOTAL GFA (4 FLOORS)	5,400 sqm	58,124 sqft
UNIT COUNT		
	15± UNITS PER FLOOR	60± UNITS TOTAL
PARKING REGULATIONS		
RESIDENTIAL USE: 1.00 SPACES PER DWELLING UNIT	60 UNITS 60 X 1.00 = 60 SPACES	60 UNITS 60 X 0.90 = 54 SPACES
RESIDENTIAL USE: VISITOR PARKING 0.25 PER UNIT	60 UNITS 60 X 0.25 = 15 SPACES	60 UNITS 60 X 0.10 = 6 SPACES
TOTAL PARKING SPACES	75 SPACES	60 SPACES
MINIMUM DRIVE ENTRANCE WIDTH		
	7.50 m	7.61 m
MINIMUM DRIVE AISLE WIDTH		
	6.00 m	6.00 m
PARKING STALL SIZE (W X L)		
	2.80 m X 6.00 m	2.80 m X 5.50 m
ACCESSIBLE STALL SIZE (W X L)		
	4.50 m X 6.00 m	4.50 m X 5.50 m
DELIVERY SPACE SIZE (W X L)		
	3.50 m X 6.00 m	3.50 m X 6.00 m

LEGEND	
	PROPERTY LINE
	PROPOSED EASEMENT
	PROPOSED BUILDING
	LANDSCAPED AREA MINIMUM LANDSCAPE OPEN SPACE 25%

SITE PLAN INFORMATION IS TAKEN FROM PLAN OF SURVEY
PREPARED BY ZUBEK, EMO PATTEN & THOMSEN LTD.
DATED: DECEMBER 12, 2022
PLAN 51R-43961
PART OF NORTH HALF OF LOT 43
CONCESSION B
(FORMERLY TOWNSHIP OF NOTTAWASAGA)
TOWN OF COLLINGWOOD
COUNTY OF SIMCOE



This drawing, as an instrument of service, is provided by and is the property of DANIEL L. CUSIMANO, ARCHITECT.

The contractor must verify and accept responsibility for all dimensions and conditions on site and must notify DANIEL L. CUSIMANO, ARCHITECT, of any variations from the supplied information.

This drawing is not to be scaled.

The architect is not responsible for the accuracy of survey, structural, mechanical, electrical, etc., information shown on this drawing. Refer to the appropriate consultant's drawings before proceeding with the work.

Construction must conform to all applicable codes and requirements of authorities having jurisdiction.

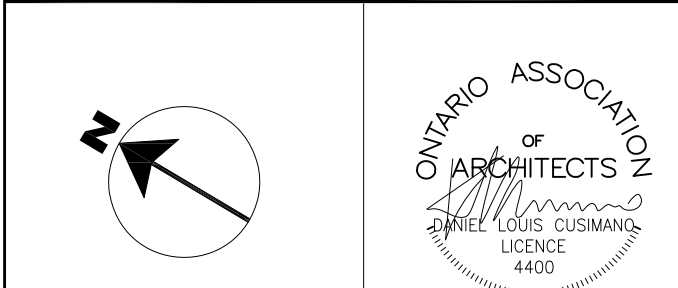
The contractor working from drawings not specifically marked 'For Construction' must assume full responsibility and bear costs for any corrections or damages resulting from his work.

PRINT DATE:

2024-04-04

No.	DESCRIPTION	DATE
REVISIONS		

2	ISSUED FOR 1ST ZONING BY-LAW AMENDMENT REVIEW	2024-04-04
1	ISSUED FOR CLIENT REVIEW	2023-08-18
No.	DESCRIPTION	DATE
ISSUED FOR		

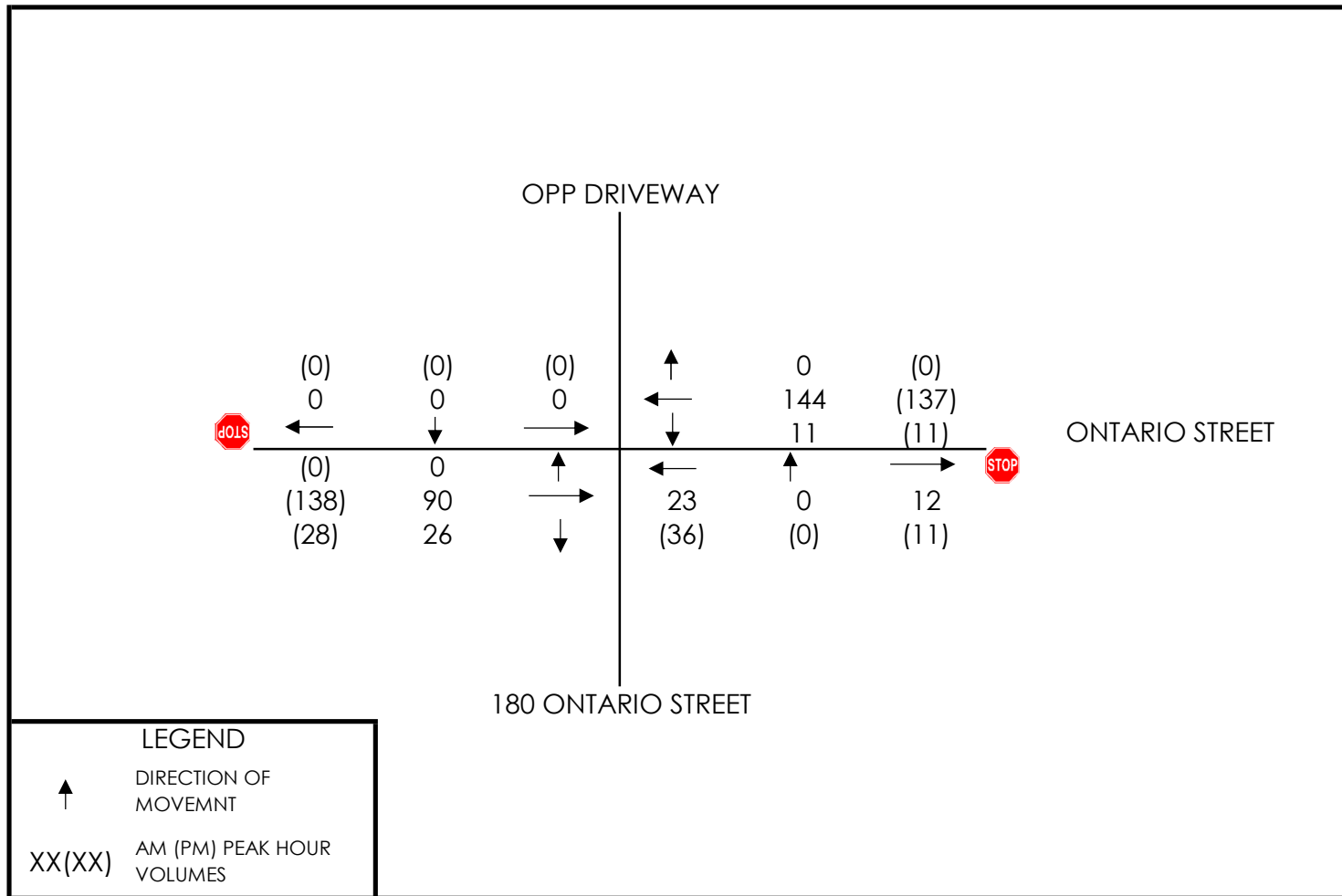


PROJECT:
**PROPOSED RESIDENTIAL
BUILDING DEVELOPMENT**
180 ONTARIO STREET
COLLINGWOOD, ON L9Y 3S5

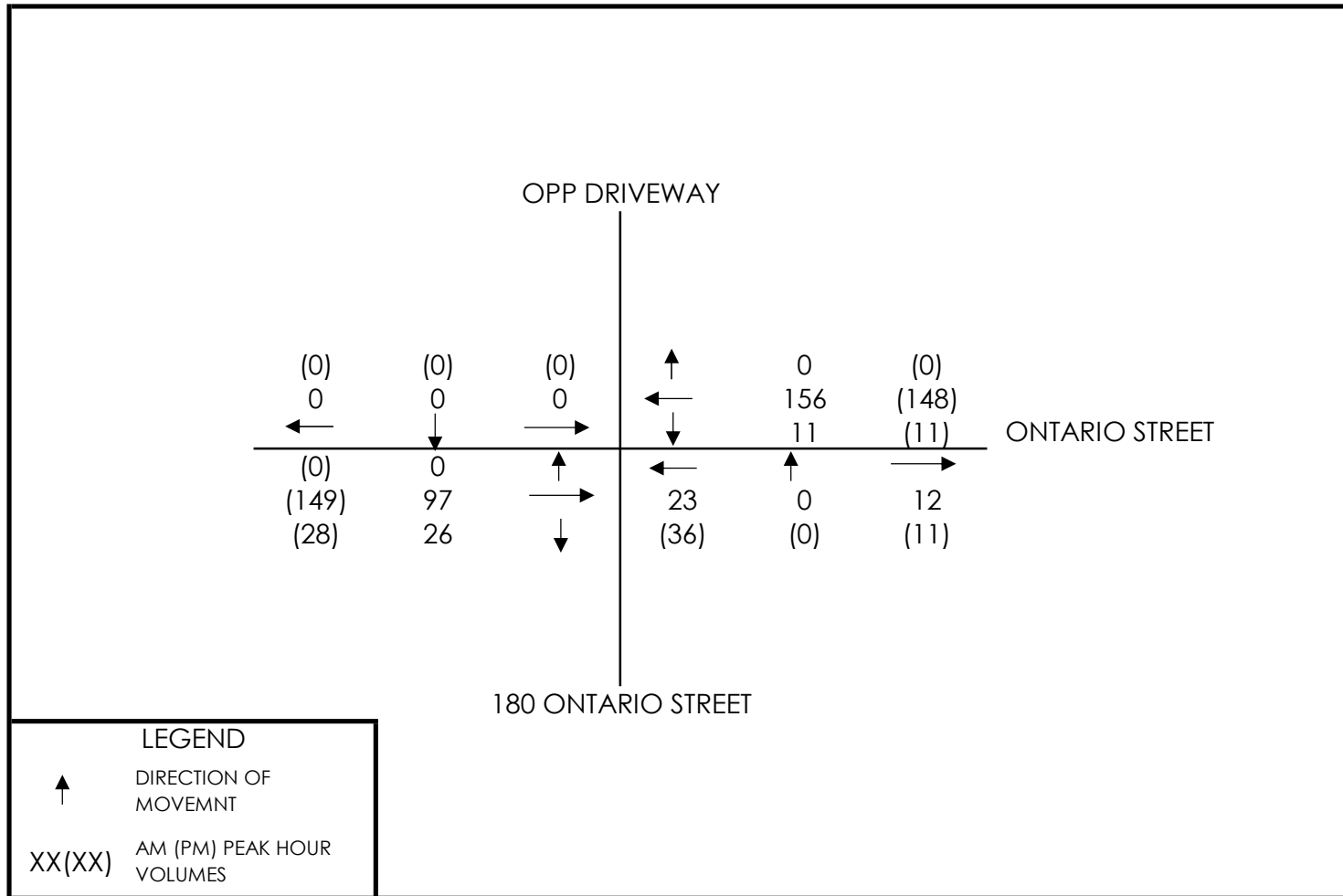
DATE: AUGUST 2023	DWN. BY: MR	CH'D. BY: DLC
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DRAWING TITLE:
PROPOSED SITE PLAN & SITE STATISTICS

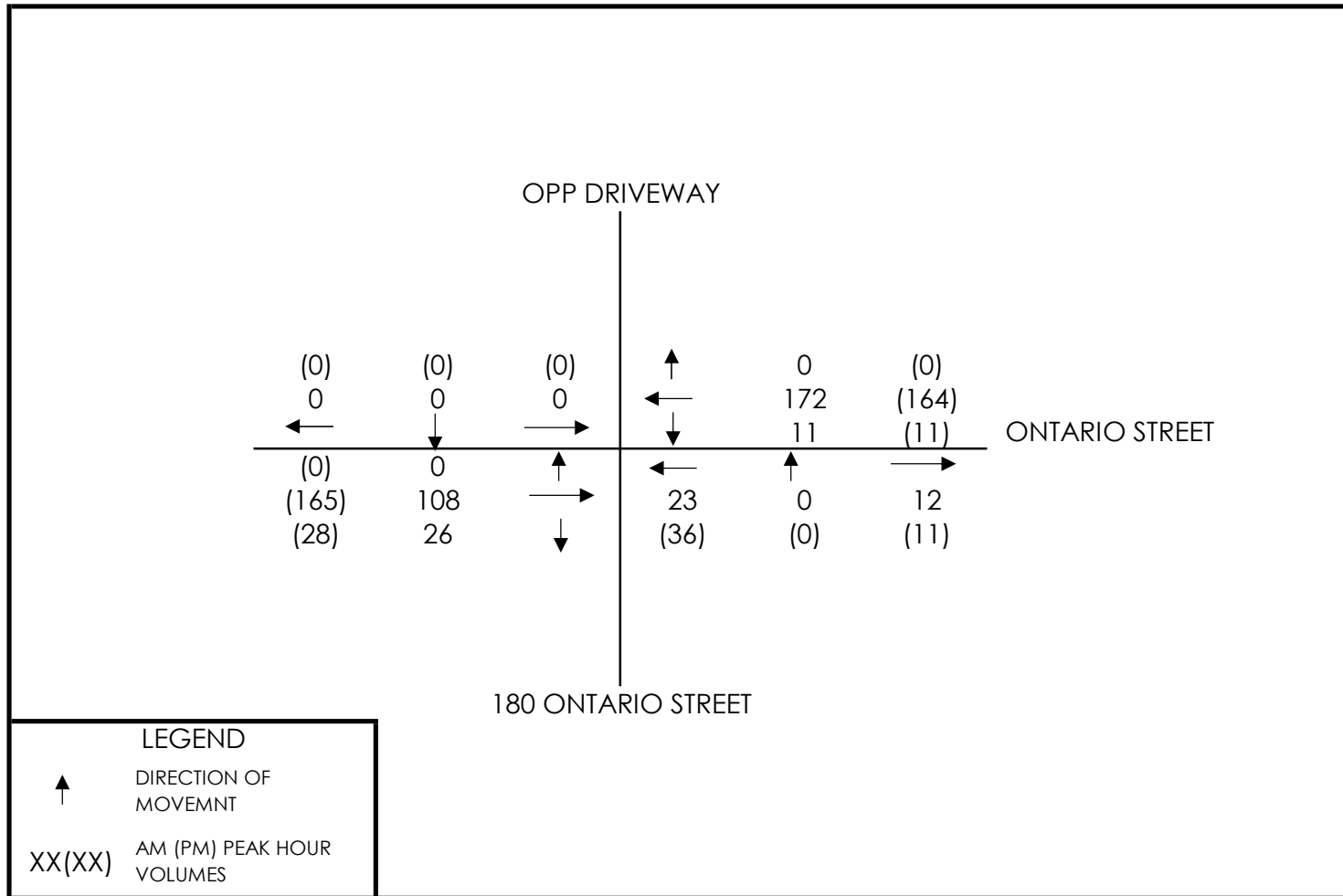
SCALE: AS NOTED	DRAWING No.: A100
PROJ. NO.: 2024-02	



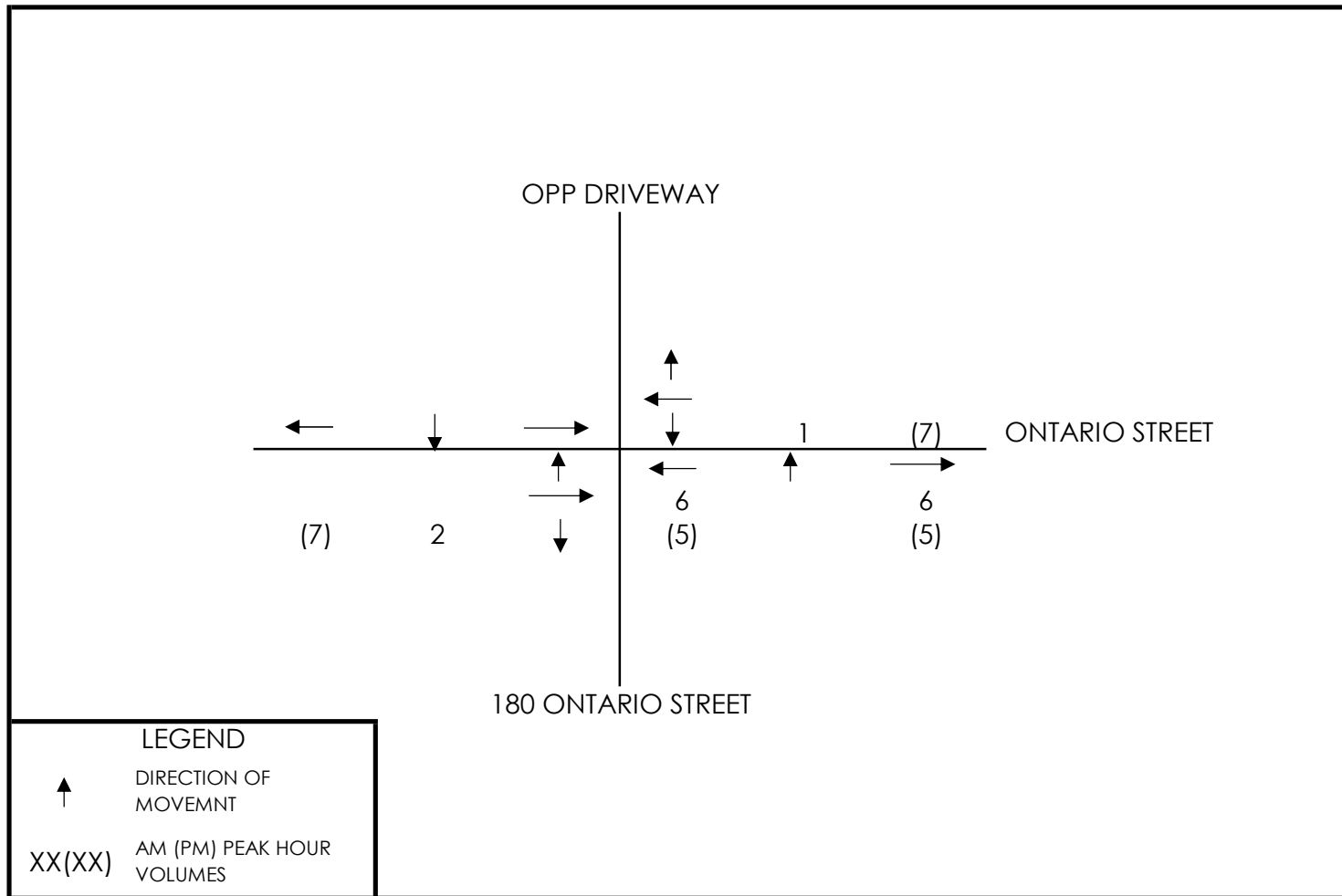
PROJECT:	180 ONTARIO STREET, COLLINGWOOD	DRAWN BY:	KH	 ADMIRAL BUILDING 1 FIRST STREET, SUITE 200 COLLINGWOOD, ON, L9Y 1A1 705-446-3510 T 705-446-3520 F WWW.CROZIER.CA INFO@CROZIER.CA	
		CHECK BY:	MF		
DRAWING:	2024 EXISTING TRAFFIC VOLUMES	SCALE:	N.T.S.	PROJECT NO. 2598-6970 FIGURE: 3	



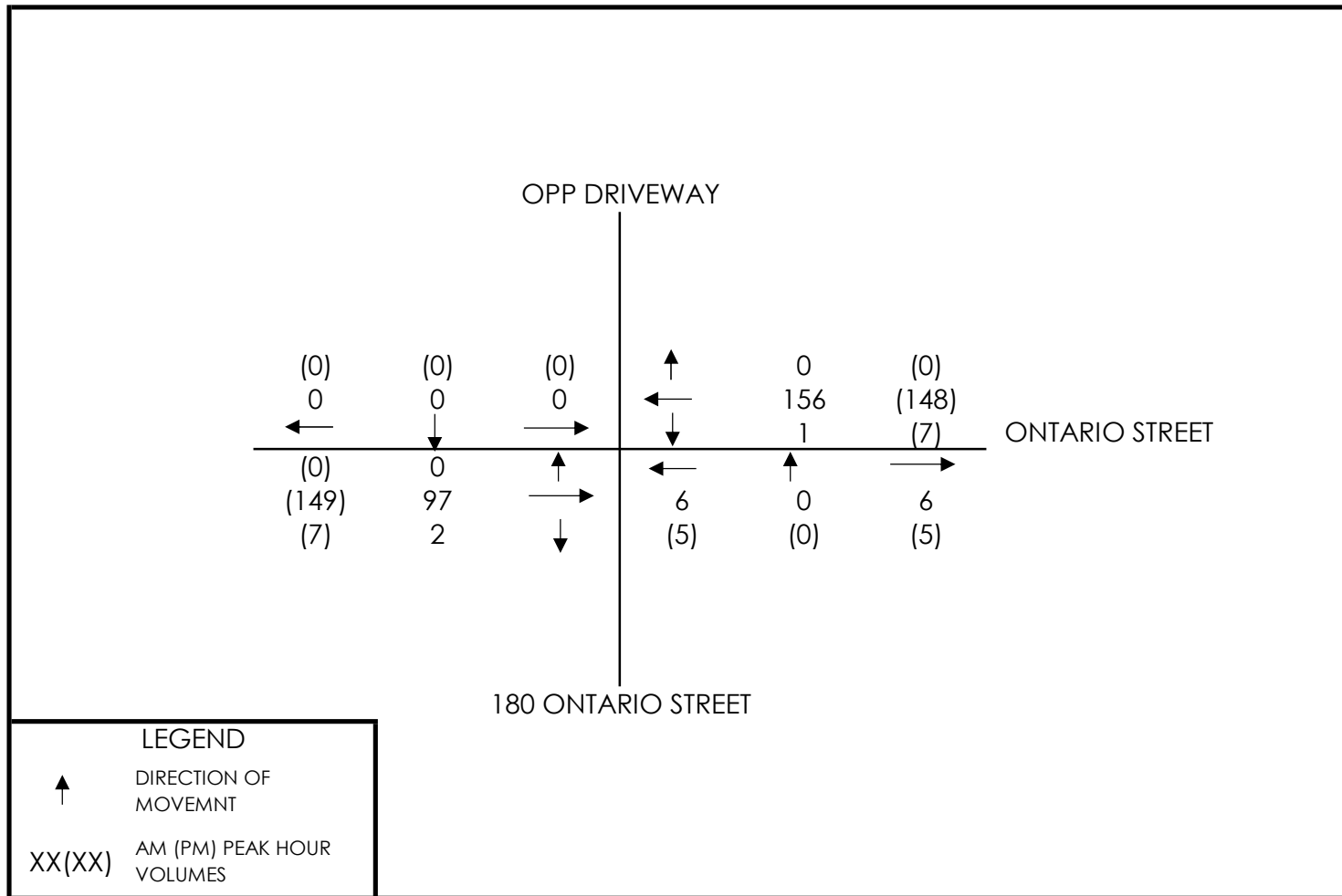
PROJECT:	180 ONTARIO STREET, COLLINGWOOD	DRAWN BY:	KH	 ADMIRAL BUILDING 1 FIRST STREET, SUITE 200 COLLINGWOOD, ON, L9Y 1A1 705-446-3510 T 705-446-3520 F WWW.CROZIER.CA INFO@CROZIER.CA	
		CHECK BY:	MF		
DRAWING:	2028 FUTURE BACKGROUND VOLUMES	SCALE:	N.T.S.		
		PROJECT NO.	2598-6970	FIGURE:	4



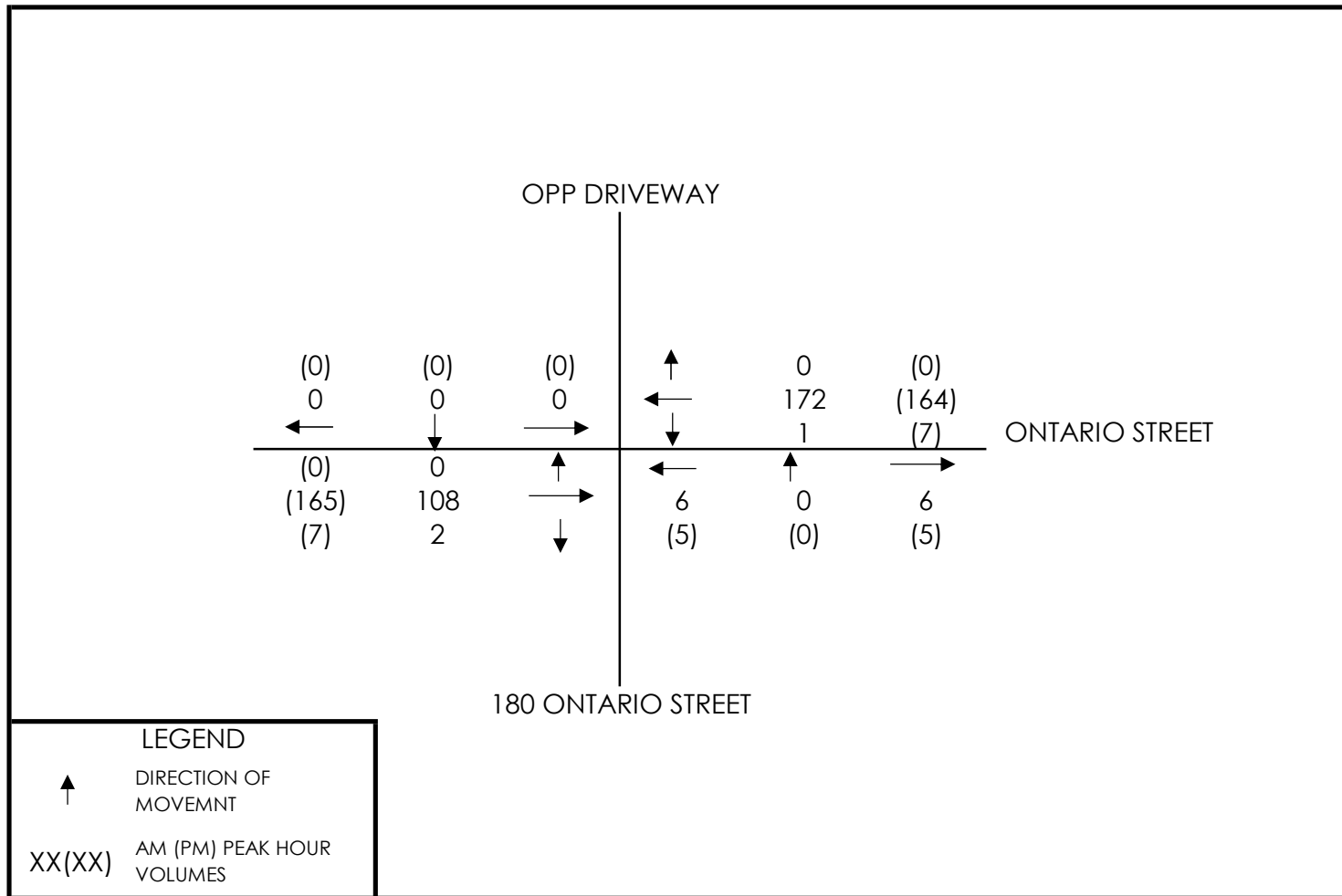
PROJECT:	180 ONTARIO STREET, COLLINGWOOD	DRAWN BY:	KH	 ADMIRAL BUILDING 1 FIRST STREET, SUITE 200 COLLINGWOOD, ON, L9Y 1A1 705-446-3510 T 705-446-3520 F WWW.CROZIER.CA INFO@CROZIER.CA	
		CHECK BY:	MF		
DRAWING:	2028 FUTURE BACKGROUND TRAFFIC VOLUMES	SCALE:	N.T.S.		
		PROJECT NO.	2598-6970	FIGURE:	5



PROJECT:	180 ONTARIO STREET, COLLINGWOOD	DRAWN BY:	KH	CROZIER CONSULTING ENGINEERS ADMIRAL BUILDING 1 FIRST STREET, SUITE 200 COLLINGWOOD, ON, L9Y 1A1 705-446-3510 T 705-446-3520 F WWW.CROZIER.CA INFO@CROZIER.CA
		CHECK BY:	MF	
DRAWING:	TRIP ASSIGNMENT	SCALE:	N.T.S.	
		PROJECT NO.	2598-6970	FIGURE: 6



PROJECT:	180 ONTARIO STREET, COLLINGWOOD	DRAWN BY:	KH	 ADMIRAL BUILDING 1 FIRST STREET, SUITE 200 COLLINGWOOD, ON, L9Y 1A1 705-446-3510 T 705-446-3520 F WWW.CROZIER.CA INFO@CROZIER.CA	
		CHECK BY:	MF		
DRAWING:	2028 FUTURE TOTAL TRAFFIC VOLUMES	SCALE:	N.T.S.	PROJECT NO. 2598-6970 FIGURE: 7	
		PROJECT NO.			

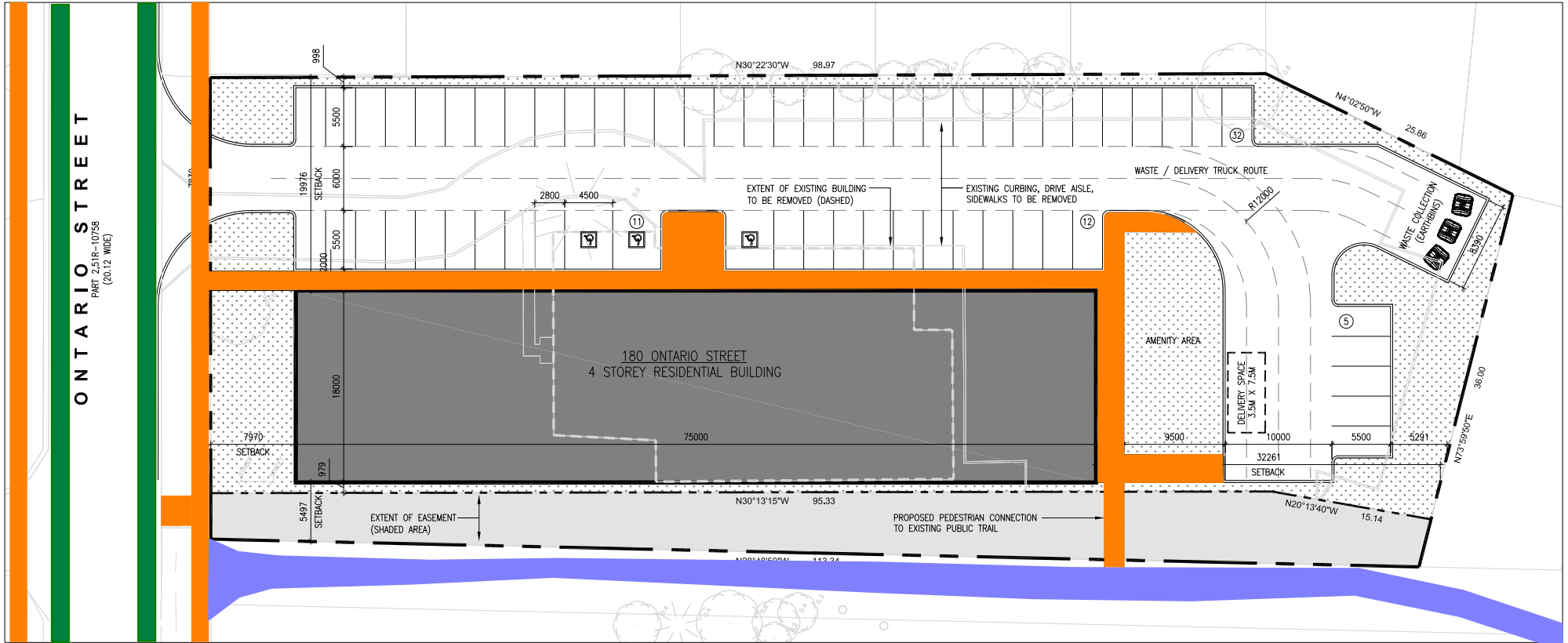
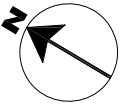


PROJECT:	180 ONTARIO STREET, COLLINGWOOD	DRAWN BY:	KH	
		CHECK BY:	MF	
DRAWING:	2033 FUTURE TOTAL TRAFFIC VOLUMES	SCALE:	N.T.S.	
		PROJECT NO.	2598-6970	FIGURE: 8



CROZIER
CONSULTING ENGINEERS

ADMIRAL BUILDING
1 FIRST STREET, SUITE 200
COLLINGWOOD, ON, L9Y 1A1
705-446-3510 T
705-446-3520 F
WWW.CROZIER.CA
INFO@CROZIER.CA



- LEGEND
- BIKE LANES
 - TRAIN TRAIL
 - SIDEWALK

Project	180 ONTARIO STREET		
Drawing	PEDESTRIAN AND CYCLING CIRCULATION PLAN		



CROZIER
CONSULTING ENGINEERS

ADMIRAL BUILDING
1 FIRST STREET, SUITE 200
COLLINGWOOD, ON L9Y 1A1
705 446-3510 T
705 446-3520 F
WWW.CFCROZIER.CA

Drawn By	KH	Design By	KH	Project	2598-6970
Scale	N.T.S.	Date	2024.02.26	Check By	MF
				Drawing	FIG. 9